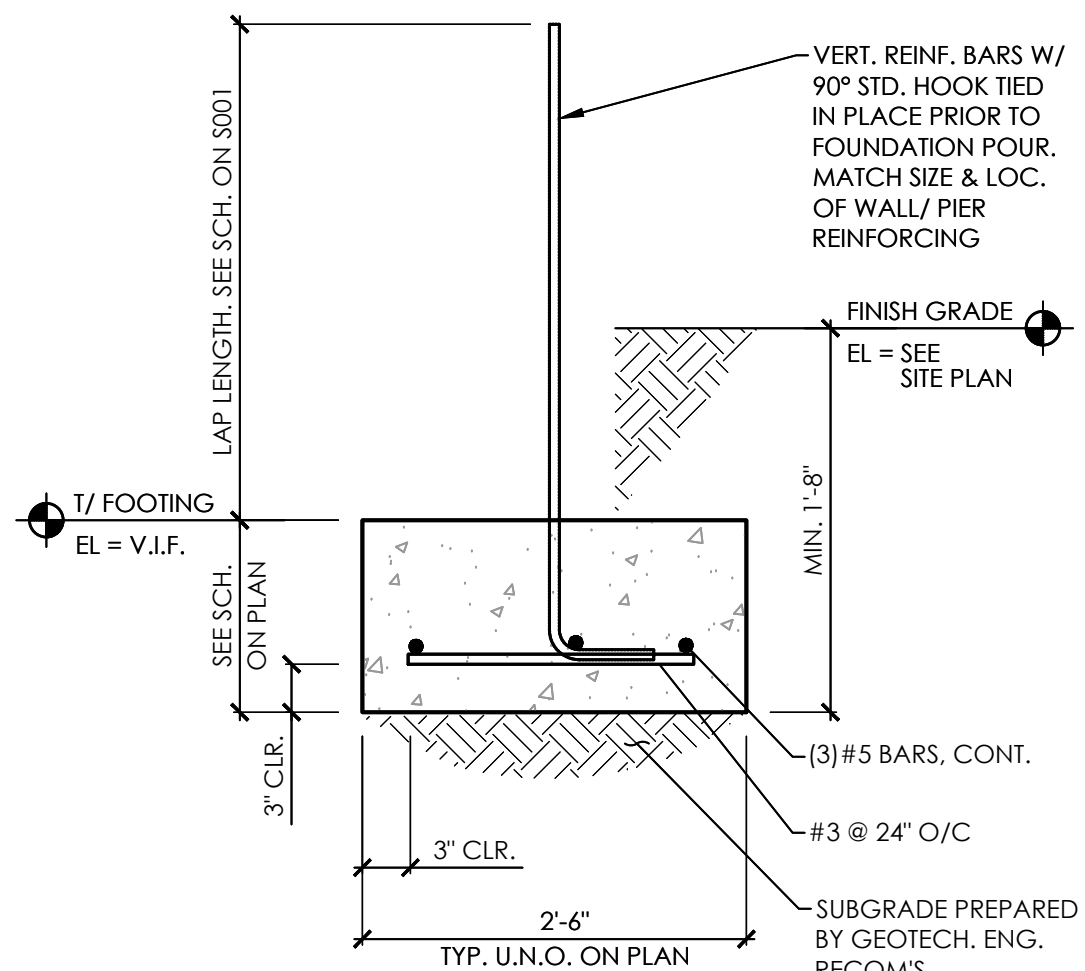
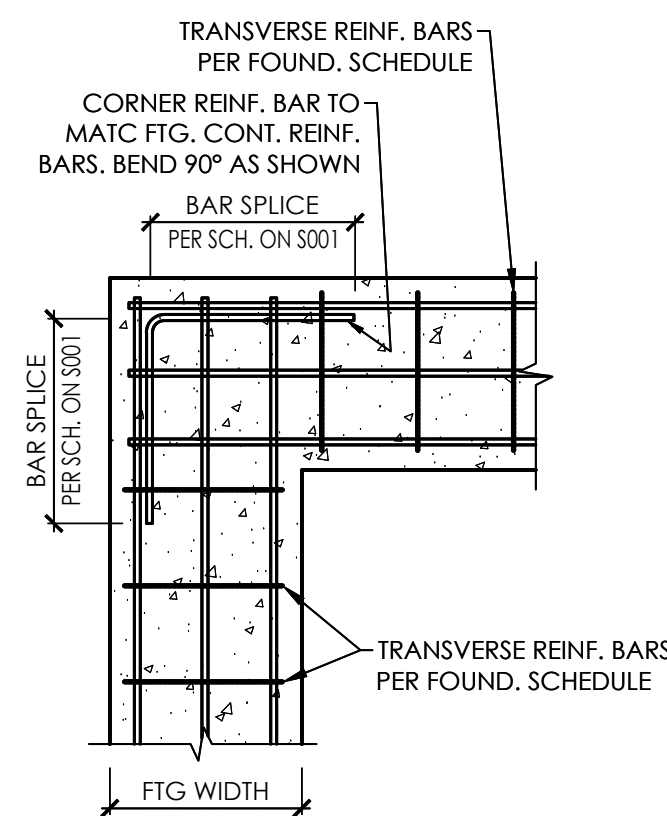


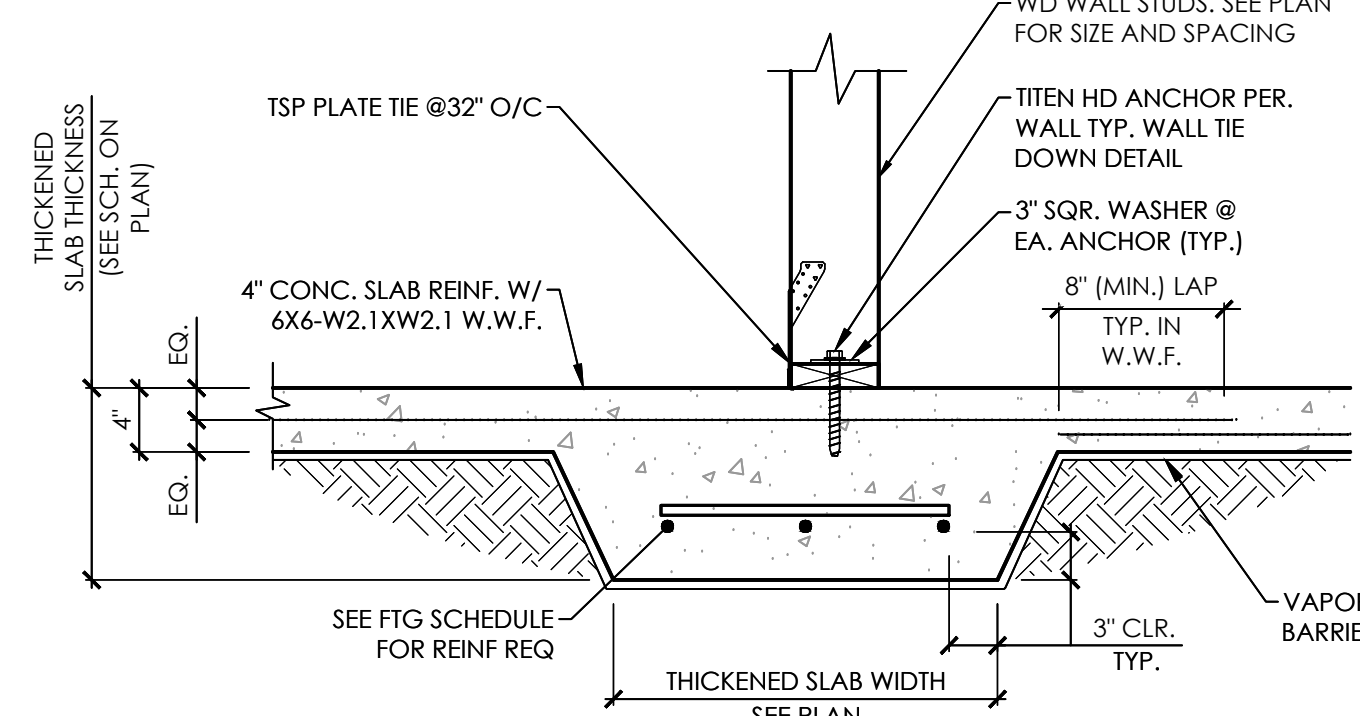
ALL REPORTS, PLANS, SPECIFICATIONS, COMPUTER FILES, FIELD DATA, NOTES AND OTHER DOCUMENTS AND INSTRUMENTS PREPARED BY THE CONSULTANT AS INSTRUMENTS OF SERVICE SHALL REMAIN THE PROPERTY OF THE CONSULTANT. THE CONSULTANT SHALL RETAIN ALL COMMON LAW, STATUTORY AND OTHER RESERVED RIGHTS, INCLUDING THE COPYRIGHT THEREIN, PURSUANT TO FLORIDA STATUTES, SECTION 338.003, AN INDIVIDUAL EMPLOYEE OR AGENT THAT MAY BE HELD INDIVIDUALLY LIABLE FOR NEGLIGENCE.



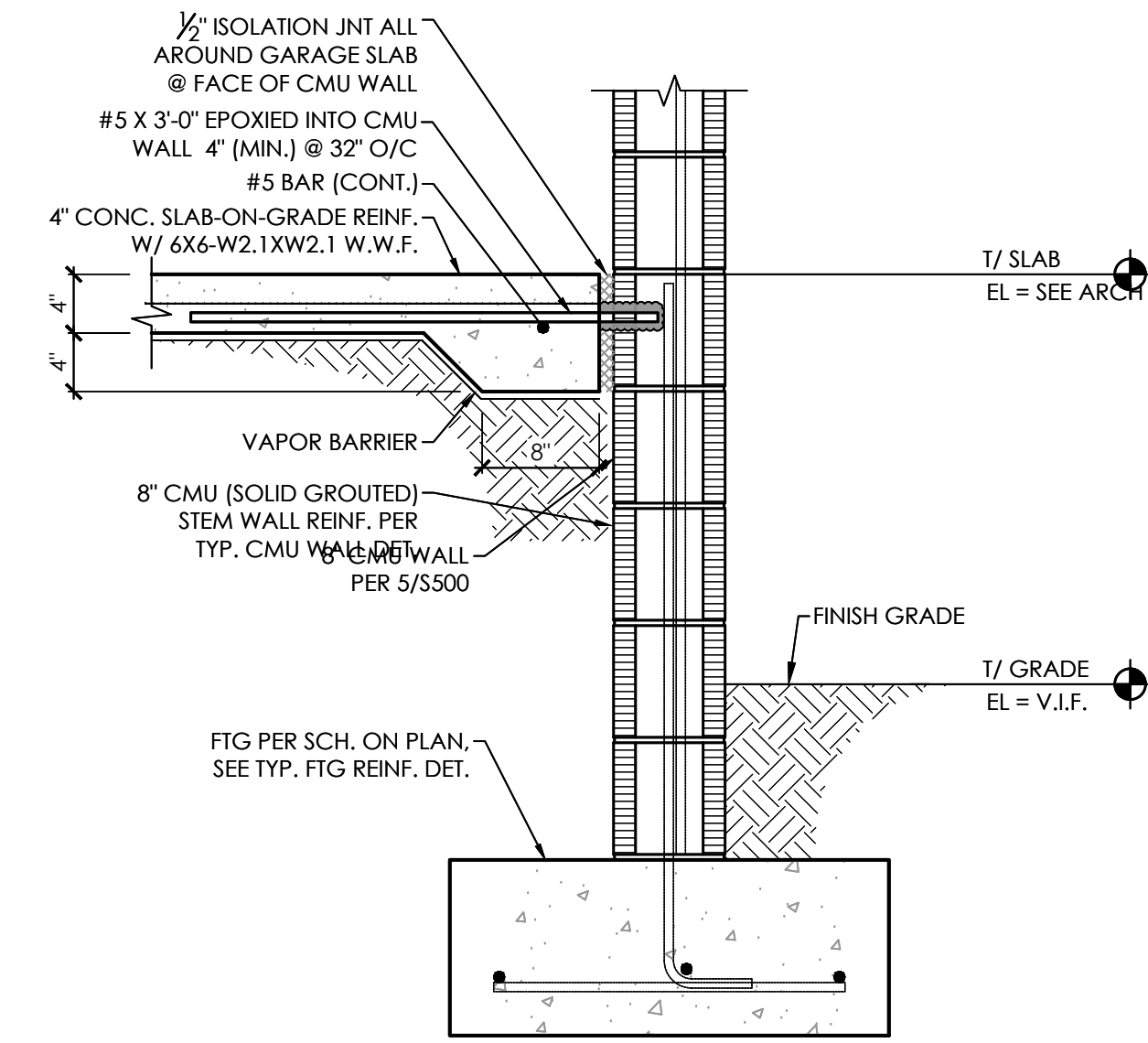
**1 TYP. CMU WALL FOUNDATION DETAIL**  
SCALE: 1"=1'-0"



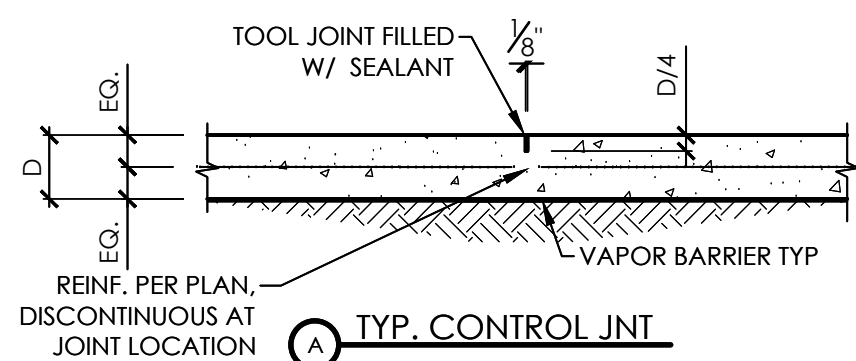
**2 TYP. CONT. FOOTING REINFORCING DETAIL @ CORNERS (PLAN VIEW)**  
SCALE: N.T.S.



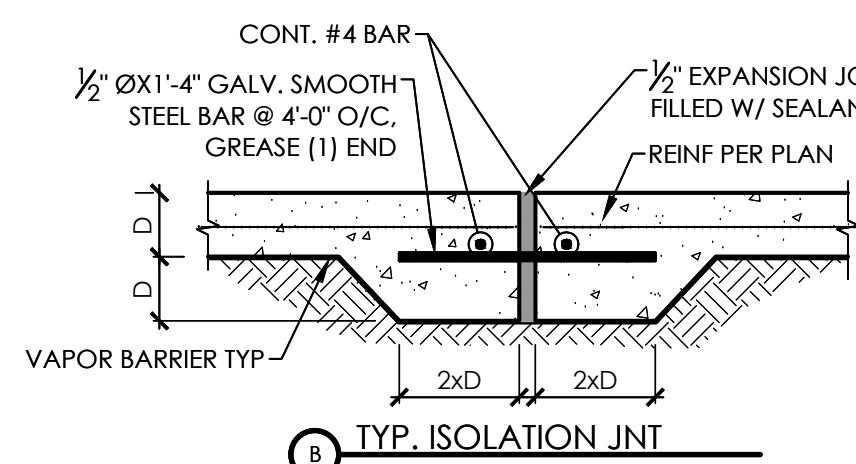
**3 TYP. SLAB ON GRADE DETAILS**  
SCALE: 1"=1'-0"



**4 TYP. SLAB POURED AGAINST CONT. STEM WALL**  
SCALE: 1"=1'-0"

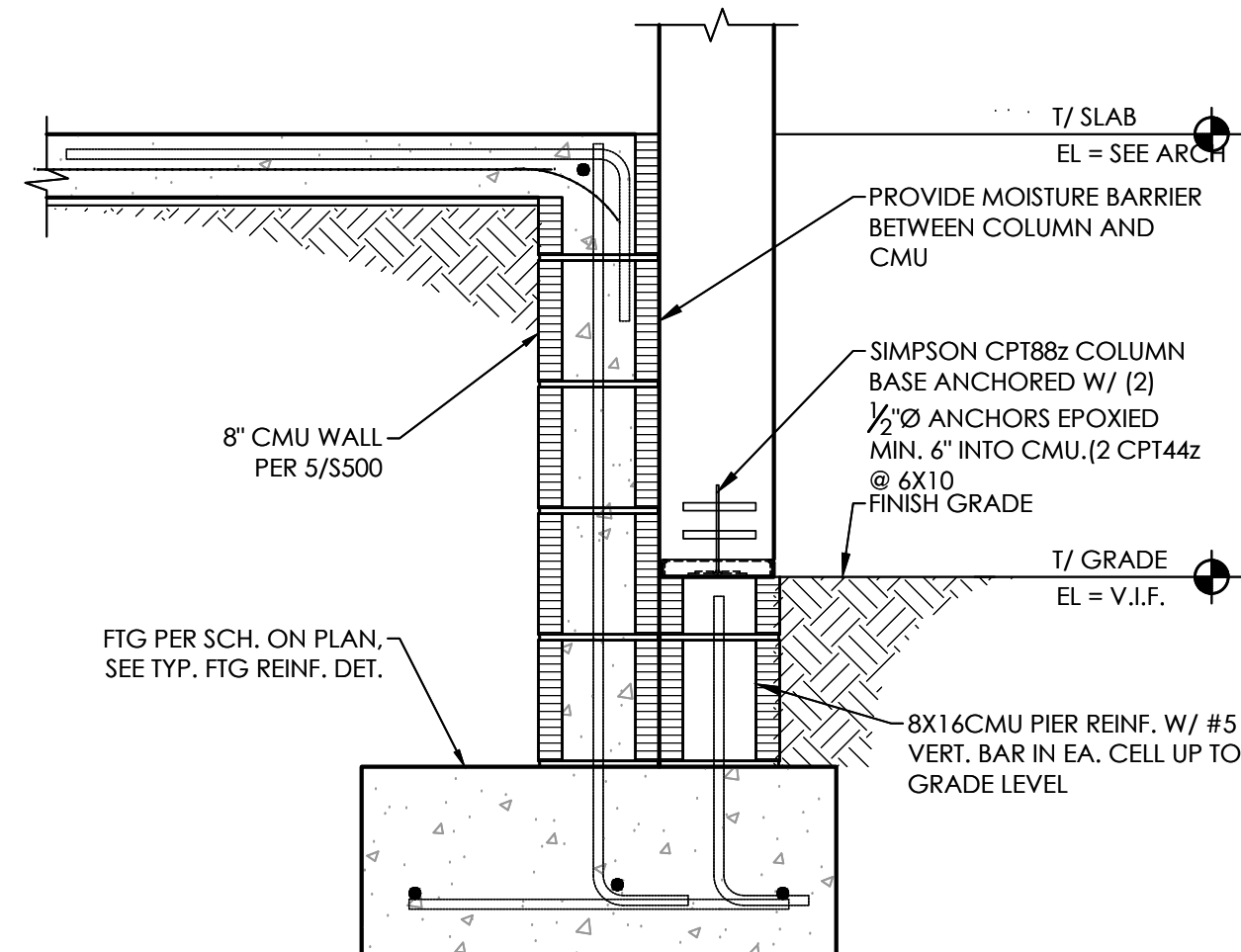


- NOTE:**
1. SPACE CONTROL JOINTS AT 10'-0" O.C. MAX EA. WAY
  2. SLAB SHALL BE SAWN AS SOON AS THE CONCRETE WILL SAFELY SUPPORT MEN AND EQUIPMENT
  3. SLAB SHALL BE SAWN WITHIN THE SAME DAY AFTER CONC. PLACEMENT
  4. SLAB SECTION BETWEEN CONTROL JOINTS SHALL HAVE WIDTH TO LENGTH RATION NOT TO EXCEED 1:1.5.
  5. SUPPORT SOILS TO BE PREPARED PER GEOTECH. REPORT

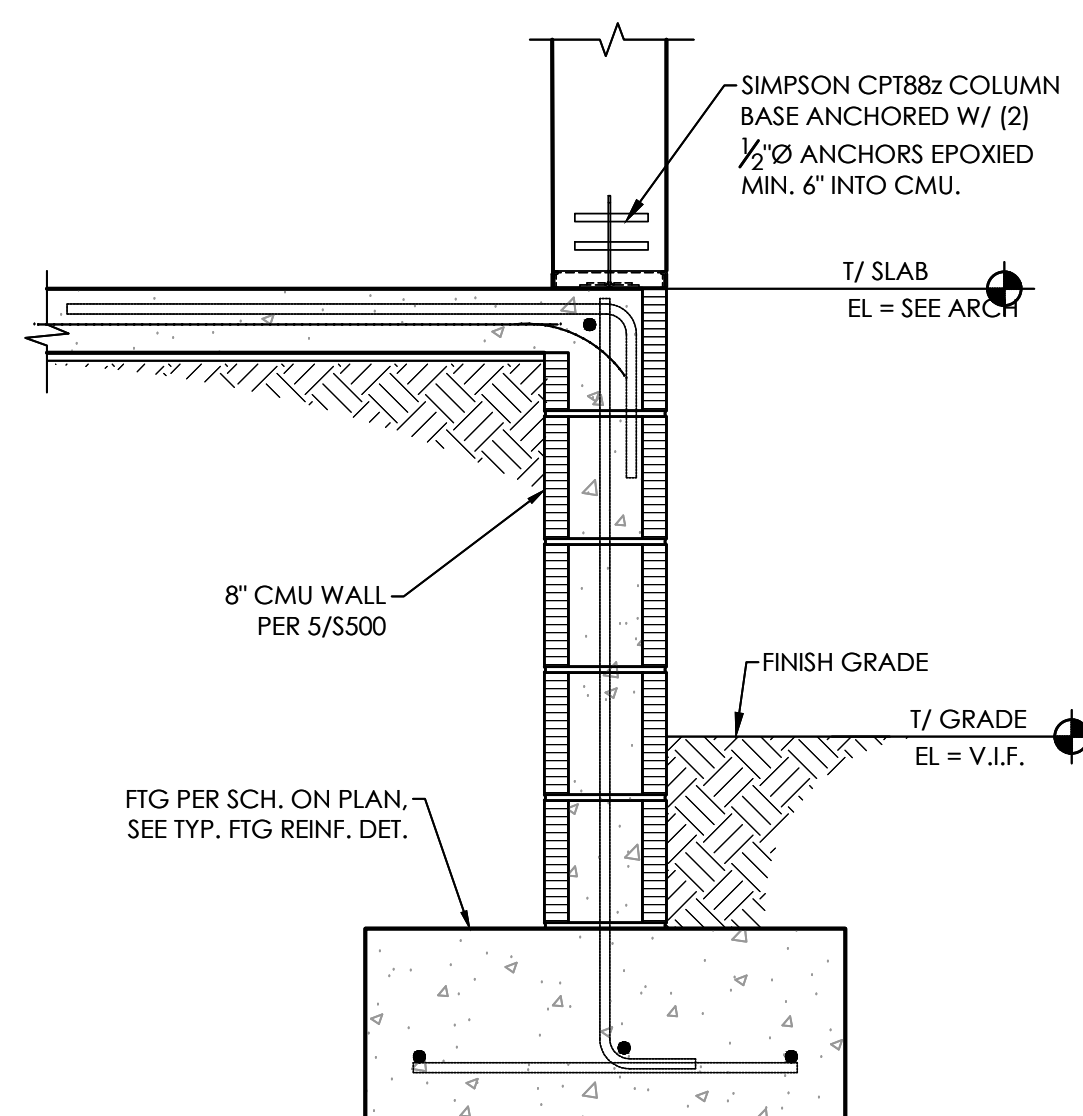


- NOTE:**
1. CONSTRUCTION JOINTS ARE REQUIRED AT ALL LOCATIONS OF DISCONTINUOUS SLAB POURS
  2. ALL SURFACES IN CONTACT WITH FUTURE CONC. PLACEMENTS SHALL BE CLEANED BEFORE PLACEMENT OF ADJACENT CONCRETE.

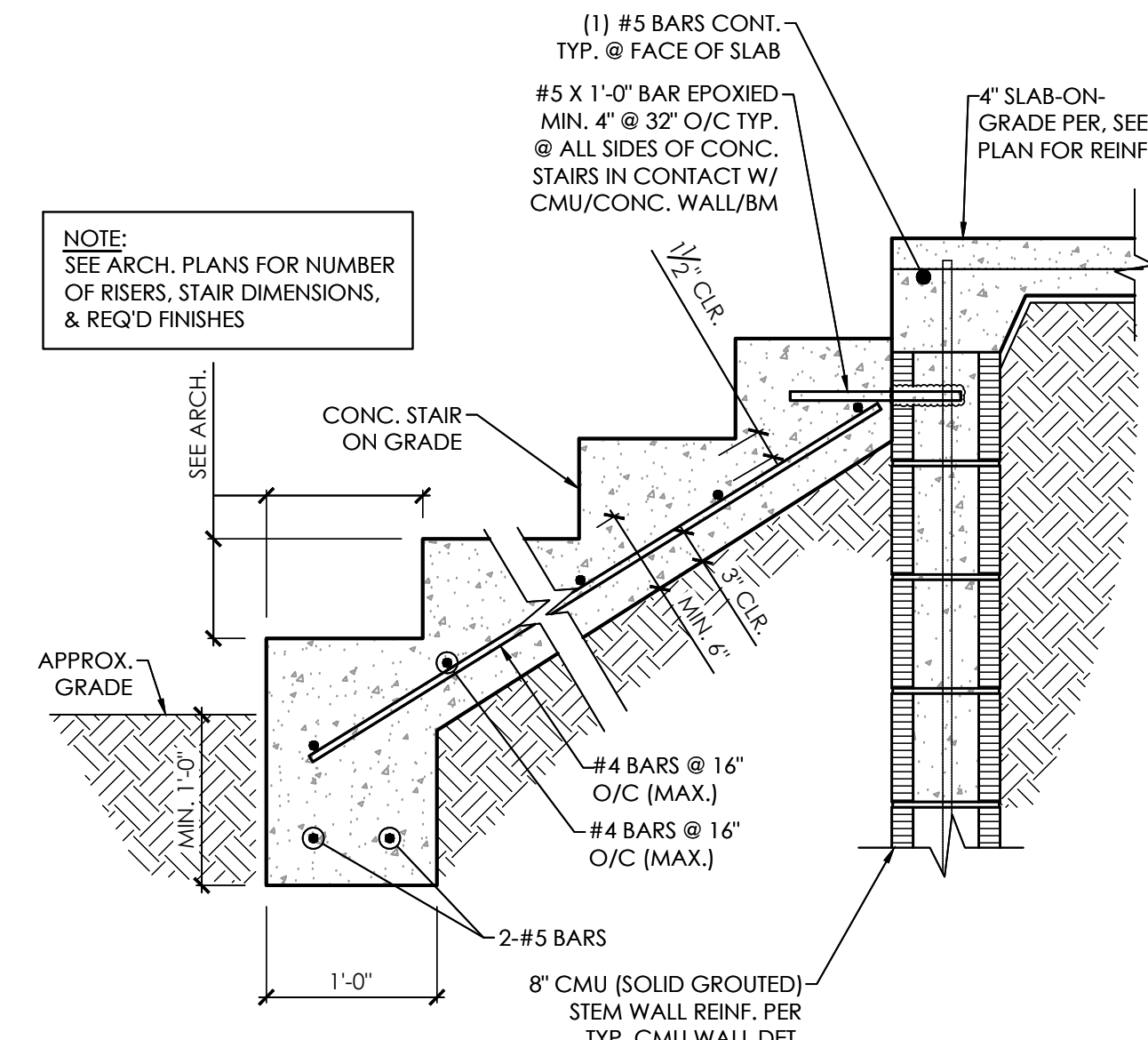
**5 TYP. SLAB JOINT DETAILS**  
SCALE: 1"=1'-0"



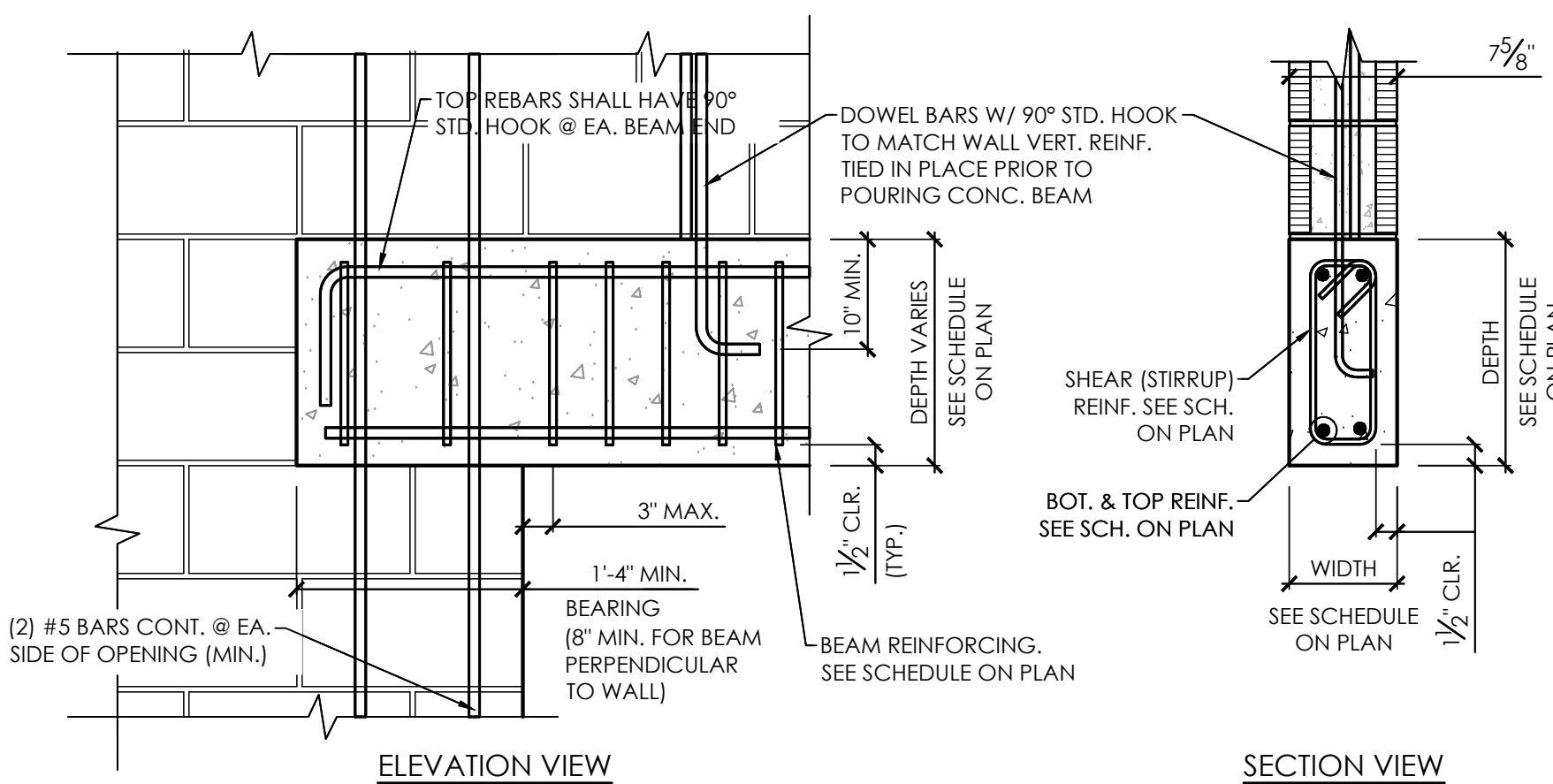
**6 TYP. EXT. WALL FOUNDATION DETAIL**  
SCALE: 1"=1'-0"



**7 TYP. EXT. WALL FOUNDATION DETAIL**  
SCALE: 1"=1'-0"



**8 TYP. CONC. STAIR ON GROUND DETAIL**  
SCALE: 1"=1'-0"



**9 TYP. C.I.P. CONC. LINTEL REINFORCING DETAIL**  
SCALE: 1"=1'-0"

**MASONRY (CAST-CRETE) LINTEL SCHEDULE**

MARK	CAST-CRETE TYPE DESIGNATION	SIZE (NOM.)		HORIZONTAL REINFORCING		LAYOUT
		CAST-CRETE LINTEL	NUMBER OF SOLID GROUTED CMU BLOCKS ABOVE	BOTTOM BARS	TOP BARS	
ML8X8	8F8-1B/OT	8X8	NOT REQ'D	1-#5	NOT REQ'D	
ML8X16	8F16-2B/OT	8X8	(118" CMU	1-#5	2-#5 NOTE: PROVIDE 1/2" SIDE CLR.	

**NOTE:**

1. ALL LINTELS SHALL HAVE 8" MIN. BEARING @ EA. END
2. ALL LINTELS SHALL BE FILLED W/ 3000 PSI GROUT MAX. 3/4" AGGREGATE & 8"-11" SLUMP.
3. DESIGN IS BASED ON CAST-CRETE PRECAST & PRESTRESSED LINTEL TYPES.
4. LINTELS THAT COVER SPAN 8'-0" AND GREATER SHALL BE PROVIDED WITH TEMPORARY SUPPORT AT MIDPOINT UNTIL GROUTED CONCRETE HAS REACHED FULL STRENGTH.

**3000 PSI CONC. BEAM SCHEDULE**

MARK	SIZE (NOM.)		HORIZ. REINF.			SHEAR REINF.	
	DEPTH (MIN., V.J.F.)	WIDTH (MIN., V.J.F.)	BOT.	SIDE	TOP	SIZE	SPACING "S"
CB1	1'-4"	8"	2-#5	-	2-#5	#3 CLOSED STIRRUP	12" O/C
CB2	1'-4"	8"	2 LAYERS	-	2-#5	#3 CLOSED STIRRUP	12" O/C



DAG ARCHITECTS  
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1223 Airport Road Destin, Florida 32541  
850.837.8152 www.dagarchitects.com

PRELIM SET

South Walton County Mosquito Control District  
New Headquarters Building

774 North County Highway 393  
Santa Rosa Beach, Florida 32459

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED  
BY: NEILL O'CONNELL, P.E.  
ON THE DATE SHOWN ON THE SIGNATURE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

PRELIMINARY  
NOT FOR CONSTRUCTION

STRUCTURAL DETAILS

No.	Description	Date

**O'CONNELL & ASSOCIATES**  
CONSULTING ENGINEERS, LLC.

CERT. OF AUTH. #30549  
1394 Co. Hwy. 283 S. Bldg. #3 Santa Rosa Beach FL 32459  
(850) 403-4555 www.oconnellengineers.com

S501

OA Project number: 18178  
DAG Project number: 18106  
Date: 08.02.23  
PIC: NOC  
PM: JMB

# STRUCTURAL GENERAL NOTES

## ABBREVIATIONS

AB	ANCHOR BOLT
AFB	ABOVE FINISH FLOOR
ARCH	ARCHITECTURAL
<b>B</b>	
BLDG	BUILDING
BM	BEAM
BOT	BOT
BRG	BEARING
<b>C</b>	
CANT	CANTILEVERED
CIP	CAST IN PLACE
CJ	CONTROL JOINT
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
COORD	COORDINATE
<b>D</b>	
DM	DIMENSION
DL	DEAD LOAD
DN	DOWN
DWGS	DRAWINGS
<b>E</b>	
EA	EACH
ECS	ELEVATED CONCRETE SLAB
ECH	EACH END
EJ	EXPANSION JOINT
EL	ELEVATION
ELV	ELEVATOR
EOS	EDGE OF SLAB
EQU	EQUAL
EQUIP	EQUIPMENT
EXIST	EXISTING
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
<b>F</b>	
FFE	FINISH FLOOR ELEVATION
FLR	FLOOR
FRT	FIRE RETARDANT TREATED
FTG	FOOTING
<b>G</b>	
GA	GAUGE
GCALV	GALVANIZED
GC	GENERAL CONTRACTOR
GLU	GLUED LAMINATED LUMBER
GT	GIRDER TRUSS
<b>H</b>	
HK	HOOK
HORIZ	HORIZONTAL
HS	HIGH STRENGTH
HT	HEIGHT
<b>I</b>	
INT	INTERIOR
<b>J</b>	
JBE	JOIST BEARING ELEVATION
JB	JOINT
<b>L</b>	
LB	POUNDS
LCS	LIGHT GAGE STEEL
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
LVL	LAMINATED VENEER LUMBER
LWL	LIGHT WEIGHT
<b>M</b>	
MAS	MASONRY
MAX	MAXIMUM
MECH	MECH
MFR	MANUFACTURER
MISC	MISCELLANEOUS
MIN	MINIMUM
ML	MASONRY LINTEL
MS	MAT SLAB
<b>N</b>	
NO	NUMBER
NIC	NOT IN CONTACT
NTS	NOT TO SCALE
NW	NORMAL WEIGHT
<b>O</b>	
OC	ON CENTER
OPP	OPPOSITE
OH	OPPOSITE HAND
CWSJ	OPEN WEB STEEL JOIST
<b>P</b>	
PDF	POWDER DRIVEN FASTENER
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PFR	FOR WOOD, PRESSURE TREATED
PT	FOR CONC. SLAB, POST-TENSIONED
<b>R</b>	
RC	REINFORCED CONCRETE
REF	REFERENCE
REINF	REINFORCING
REQD	REQUIRED
<b>S</b>	
SIM	SIMILAR
SOG	SLAB-ON-GRADE
SP	SOUTHERN PINE
SPA	SPACE
STD	STANDARD
STIFF	STIFFENER
<b>T</b>	
TBD	TO BE DETERMINED
TBE	TRUSS BEARING ELEVATION
T&B	TOP & BOTTOM
T&G	TONGUE AND GROOVE
TS	TOP OF SLAB/STEEL / SUBFLOOR
TOS	THICKENED SLAB ON GRADE
TP	TYPICAL
<b>U</b>	
UNO	UNLESS NOTED OTHERWISE
<b>V</b>	
VERT	VERTICAL
<b>W</b>	
WB	WOOD BEAM
WC	WOOD COLUMN
WCJ	WALL CONTROL JOINT
WHC	WOOD HOLLOW (BOX) COLUMN
WSP	WOOD STUD PACK/ BUILT-UP COLUMN
WT	WEIGHT
<b>MISC</b>	
[H]	HIGH
[L]	LOW

## GENERAL

- NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, ENGINEER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS, NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE STRUCTURAL ENGINEER OF RECORD OR ANY OF THE STRUCTURAL ENGINEER OF RECORDS CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
- CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
- REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
- CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF A.C.I., P.C.I., A.S.C., S.I. OR OTHER STANDARDS, WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
- MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
- CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.
- CONTRACTOR SHALL OBTAIN AND COORDINATE EDGE OF SLAB DIMENSIONS, OPENING LOCATIONS AND DIMENSIONS, DECREASED SLAB LOCATIONS AND EXTENTS, SLAB SLOPES, CURB LOCATIONS, AND CMU WALL LOCATIONS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
- CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY.
- CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONTRACTIBILITY ANALYSIS, AND ENFORCEMENT PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
- CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
- REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR.
- SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. SHOP DRAWINGS SHALL BE SEALED BY ENGINEER LICENSED IN THE PROJECT STATE. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT/STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT/STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. SHOP DRAWINGS SHALL BE SEALED BY ENGINEER LICENSED IN PROJECT STATE.
- WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION, IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR. THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.

## CODE DESIGN

- STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2020 FLORIDA RESIDENTIAL BUILDING CODE.
- GRAVITY LOADS
  - UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
 

LOBBIES	100 PSF
OFFICES	50 PSF
STAIRS AND EXITS	100 PSF
HANDRAIL/GUARDRAIL	50 PLF (200 LBS POINT LOAD)
GUARD RAIL	50 LBS
PASSENGER VEHICLE GARAGES	50 PSF (2000 LBS POINT LOAD)
MECHANICAL MEZZANINE	200 PSF
UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):	
ROOF	20 PSF
- WIND LOADS: SEE TABLE
- ESTIMATED DEFLECTIONS (IN INCHES) ARE AS FOLLOWS:
 

	LIVE LOAD	DEAD + LIVE LOAD
ROOF MEMBERS: 1/360 OR 1 IN.	L/240	L/360
FLOOR MEMBERS: 1/480	L/360	L/480

WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN CENTERLINES OF SUPPORTS. (FOR CANTILEVERS, IT IS TWICE THE LENGTH OF THE CANTILEVER.)

## FOUNDATION

- FOUNDATION AND SLAB ON GRADE DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF RECOMMENDED IN THE GEOTECHNICAL ENGINEERING REPORT PREPARED BY NOVA ENGINEERING AND ENVIRONMENTAL NOVA PROJECT NUMBER 10111-2022(11) DATED JUNE 21, 2022.
- STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.
- FOR INITIAL SITE PREPARATION, FOUNDATION EXCAVATION, AND FILL PLACEMENT RECOMMENDATIONS. SEE THE GEOTECHNICAL ENGINEERING REPORT PREPARED BY NOVA ENGINEERING AND ENVIRONMENTAL.
- NO FOOTINGS SHALL BE PLACED IN WATER.
- BACK FILL BOTH SIDES OF FOUNDATION WALLS AT SAME TIME TO PREVENT OVERTURNING.

## REINFORCEMENT

- REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND HAVE MINIMUM SIDE AND END LAPS OF 6".
- REINFORCING SHALL NOT BE WELDED NOR TACK-WELDED UNLESS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. IF APPROVED, REINFORCING STEEL SHALL ONLY BE WELDED OR TACK-WELDED IN ACCORDANCE WITH ASTM A706, AND AWS D1.4.
- SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZE AND PLACEMENT, WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATION, AND DETAILS IS NOT ACCEPTABLE.
- PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE. REINFORCING DOWELS MUST BE TIED IN PLACE PRIOR TO POURING FOOTING. "WEI-STICKING" IS NOT ALLOWED.
- PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
  - CAST-IN-PLACE (NON POST-TENSIONED) CONCRETE REINFORCEMENT COVER
 

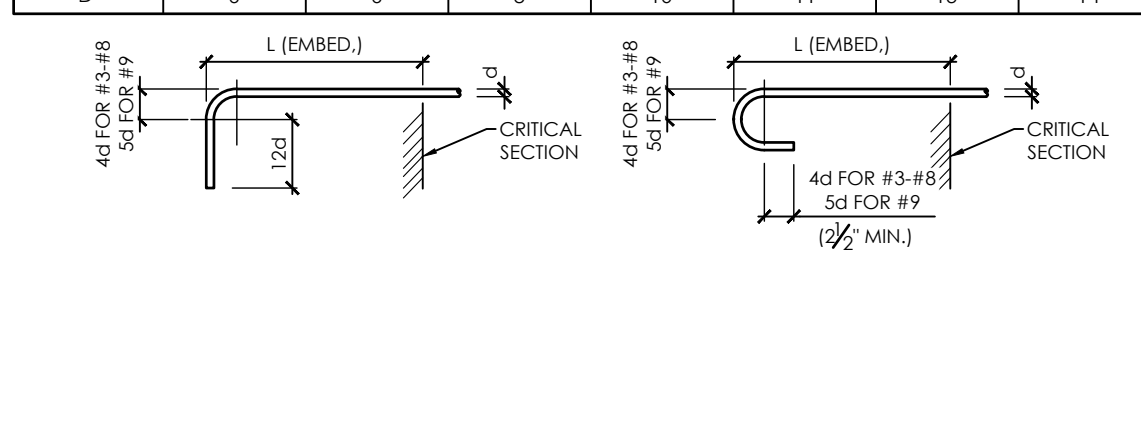
PERMANENTLY EXPOSED TO EARTH:	3" CLEAR
CAST AGAINST THE EARTH	3" CLEAR
EXPOSED TO EARTH OR WEATHER:	2" CLEAR
FOR BARS LARGER THAN NO. 5 BAR	2 1/2" CLEAR
NO. 5 BARS OR SMALLER	1 1/2" CLEAR
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH (INTERIOR STRUCTURES ONLY):	1" CLEAR
NO. 14 & NO. 18	1 1/2" CLEAR
NO. 11 & SMALLER	1" CLEAR

MINIMUM LAP SPlice LENGTH (IN.)								
	No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	
3000 PSI CONC.	21	28	36	43	52	61	70	80
4000 PSI CONC.	18	25	31	37	44	52	61	70

MINIMUM LAP SPlice LENGTH (IN.) - 1500 PSI NORMAL WEIGHT CMU								
	No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	
8-in CMU WALL	19	25	31	37	44	52	61	70
12-in CMU WALL	19	25	31	37	44	52	61	70

RECOMMENDED END HOOKS EMBEDMENT LENGTH (IN.)								
	No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	
D	6	6	8	10	11	13	14	



## POST INSTALLED ANCHORS

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID COLLISION WITH EXISTING REBAR. HOLES SHALL BE DRILLED TO THE DEPTH AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS READ BY THE BUILDING CODE. PROVIDE CONTRACT SPECIAL INSPECTION FOR THE MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE EVALUATION REPORT. CONTACT MANUFACTURER'S REPRESENTATIVE FOR THE INITIAL TRAINING AND INSTALLATION OF ANCHORS AND FOR PRODUCT RELATED QUESTIONS AND AVAILABILITY.
  - CONCRETE ANCHORS
    - MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.2 AND ICC-ES AC308 FOR CRACKED AND UNCRACKED CONCRETE RECOGNITION. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:
      - SIMPSON STRONG-TIE "TITEN HD" (ICC-ES ESR-2713)
      - SIMPSON STRONG-TIE "STRONG-BOLT Z" (ICC-ES ESR-3037)
    - ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.4 AND ICC-ES AC308 FOR CRACKED AND UNCRACKED CONCRETE RECOGNITION. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:
      - SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508)
      - HILTI HIT HY200 INJECTION ADHESIVE
- MASONRY ANCHORS
  - ANCHORAGE TO SOLID-GROUTED CONCRETE MASONRY:
    - MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR AC106. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:
      - SIMPSON STRONG-TIE "TITEN HD" (ICC-ES ESR-1056)
      - SIMPSON STRONG-TIE "STRONG-BOLT Z" (IAMPO-ES ESR-0240)
      - SIMPSON STRONG-TIE "WEDGE-ALL" (ICC-ES ESR-1981)
    - ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:
      - SIMPSON STRONG-TIE "SET-XP" (IAMPO-ES ESR-0265)
      - HILTI HIT HY200 INJECTION ADHESIVE
  - ANCHORS LISTED ABOVE SHALL NOT BE INSTALLED IN MORTAR JOINTS BETWEEN CONCRETE MASONRY UNITS.
  - ANCHORS LISTED ABOVE SHALL HAVE THE FOLLOWING MIN. EDGE DISTANCES UNLESS NOTED OTHERWISE IN STRUCTURAL DETAILS OR SPECIFICATIONS OF MOUNTED CONNECTOR/ HANGER:
    - 3" FROM TOP OF CMU WALL AND SLAB-ON-GRADE POURED OVER CMU STEM WALL;
    - 4" FROM TOP AND SIDE EDGE OF CMU WALL IF INSTALLED IN INTERIOR OR EXTERIOR FACE OF WALL;
    - 2" FROM EDGE OF CMU IF INSTALLED IN NARROW FACE OF CMU WALL.

## CAST-IN-PLACE CONCRETE

- CONCRETE WORK SHALL CONFORM TO ACI 318-14 AND CRSI STANDARDS.
- CONCRETE SHALL HAVE A MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI.
- REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
- CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. NO HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE SHALL BE SHOWN ON THE STRUCTURAL DRAWINGS.
- DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE STRUCTURAL ENGINEER.

## CONCRETE MASONRY

- CONCRETE MASONRY WORK SHALL CONFORM TO ACI 530/530.1-13, BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES.
- CONCRETE MASONRY UNITS (CMU'S) SHALL BE MEDIUM-WEIGHT, OR NORMAL-WEIGHT, CLOSED-END, CONFORMING TO ASTM C90, WITH MINIMUM COMPRESSIVE STRENGTH AT THE TIME OF INSTALLATION  $F_m = 2,000$  PSI (MIN).
- MORTAR SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND SHALL BE OF THE FOLLOWING TYPE:
 

WALLS BELOW GRADE & EXT. WALLS	TYPE M OR S
--------------------------------	-------------
- CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 3,000 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT SHALL CONFORM TO ASTM C474.
- PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 9 G-IRG TRUSS-TYPE REINFORCING WIRES AT EVERY ALTERNATIVE COURSE (AT 16" O/C), UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC.
- MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5 @ 32" UNLESS NOTED OTHERWISE.
- CMU WALLS SHALL BE:
  - AT EACH FLOOR LEVEL & TOP OF WALL @ 16" O/C MAX. DOUBLE CMU BOND BEAMS REINFORCED WITH (2) #5 BARS ON BOTTOM AND (2) #5 BARS ON TOP.
  - BETWEEN DOUBLED BOND BEAMS, SINGLE CMU BOND BEAM @ 16" O/C REINFORCED WITH (1) #5 BAR. BOND BEAMS SHALL BE CONTINUOUS. CONTINUITY SHALL BE PROVIDED BY LAPPING SPICES NOT LESS THAN 30" AND BENDING BARS AROUND CORNERS MIN. 30".
- DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENTS OF DEFECTIVE AREA TO BE DETERMINED BY THE STRUCTURAL ENGINEER.
- REINFORCING DOWELS MUST BE TIED IN PLACE PRIOR TO POURING FOOTING. "WEI-STICKING" IS NOT ALLOWED.
- GROUT LIFT SHALL NOT EXCEED:
  - 4 FT FOR 8" CMU WALL
  - 8 FT FOR 8" CMU WALL
  - 12 FT FOR 12" CMU WALL

## WOOD

- ALL WOOD FRAMING INCLUDING TRUSSES SHALL CONFORM TO THE LATEST EDITIONS OF AMERICAN INSTITUTE OF TIMBER CONSTRUCTION PUBLICATIONS AND STANDARDS.
- PROVIDE DRESSED SEASONED LUMBER SOUTHERN PINE NO. 2 OR BETTER, S4S, WITH A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF DRESSING.
- WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE FOUNDATION GRADE PRESURE-TREATED. USE GALVANIZED NAILS IN PRESURE-TREATED WOOD. THE PROTECTIVE COATING ON LIGHT GAUGE STEEL CONNECTIONS IN CONTACT W/ PRESURE-TREATED WOOD SHALL BE IN ACCORDANCE WITH THE CONNECTOR MANUFACTURER'S RECOMMENDATIONS.
- ENGINEERED LUMBER PRODUCTS
  - LAMINATED VENEER LUMBER (LVL) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND PROPERTIES:
 

ALLOWABLE BENDING STRESS	$F_b = 2600$ PSI
COMPRESSION PERPENDICULAR TO GRAIN	$F_{c\perp} = 750$ PSI
COMPRESSION PARALLEL TO GRAIN	$F_{c\parallel} = 2510$ PSI
HORIZONTAL SHEAR	$F_v = 285$ PSI
MODULUS OF ELASTICITY	$E = 2,000,000$ PSI

## STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- STRUCTURAL STEEL SHALL BE OF THE FOLLOWING GRADE UNLESS NOTED OTHERWISE ON DRAWINGS:
  - W-SHAPES SHALL CONFORM TO ASTM A992, GRADE 50. (ASTM A572, GRADE 50 MAY BE SUBSTITUTED FOR ASTM A992).
  - SQUARE/RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE B.
  - OTHER STEEL SHAPES (CHANNELS, ANGLES, AND PLATES) MAY CONFORM TO ASTM A36.
  - BOLTS, RODS, ANCHORS AND HEADED STUDS:
    - ALL CONNECTIONS SHALL BE TIGHTENED USING "TURN-OF-NUT" METHOD WITH MINIMUM 3/4" DIAMETER A325 HIGH-STRENGTH BOLTS.
    - ANCHOR RODS SHALL CONFORM TO ASTM A36 OR A307, UNLESS NOTED OTHERWISE.
    - HEADED STUDS SHALL BE 3/4" DIAMETER, UNLESS NOTED OTHERWISE, AND SHALL CONFORM TO AWS D1.1. LENGTH OF STUD SHALL BE AS NOTED ON THE DRAWING.
- CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.
  - STANDARD SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE-ANGLE, SINGLE PLATE, SINGLE-ANGLE, OR TEE CONNECTIONS IN ACCORDANCE WITH CONNECTION TABLES IN THE "MANUAL OF STEEL CONSTRUCTION: LRFD, SECOND EDITION, VOLUME II, PART 9.
  - BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2000.
  - FOR WELDED CONNECTIONS, USE PRE QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY. NON-PRE QUALIFIED JOINTS SHALL BE QUALIFIED PRIOR TO FABRICATION.
- STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED, UNLESS OTHERWISE DIRECTED BY THE ARCHITECT OR ENGINEER.
- SUBMIT STEEL SHOP DRAWINGS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.

## STRUCTURAL INSPECTION

- STRUCTURAL ENGINEER TO BE NOTIFIED IN WRITING ONE WEEK (IF POSSIBLE, THREE DAYS MINIMUM PRIOR TO ALL STRUCTURAL INSPECTION DATES) (SEE STRUCTURAL INSPECTION SCHEDULE BELOW). COPY ARCHITECT ON ALL CORRESPONDENCE WITH STRUCTURAL ENGINEER, INCLUDING THE SCHEDULING OF SITE INSPECTIONS. FINAL CERTIFICATE OF OCCUPANCY LETTERS ARE EXPECTED TO COME FROM THE STRUCTURAL ENGINEER, AND IT IS THEREFORE CRITICAL THAT HE/HE SHE DO ALL OF THE STRUCTURAL INSPECTIONS. THIS INCLUDES FOOTING INSPECTIONS PRIOR TO POURING CONCRETE.
- TYPICAL REQUIRED INSPECTIONS (WHERE APPLICABLE)
  - CERTIFICATION (DRIVING LOGS REQ'D)
    - CONTINUOUS OR SPREAD FOOTINGS (LAYOUT AND REINFORCING)
    - GRADE BEAM (LAYOUT AND REINFORCING)
    - STEM WALL BLOCK AND/OR BLOCK PIER (LAYOUT AND REINFORCING)
  - FLOORS, ROOFS, AND WALLS:
    - FLOOR/ ROOF SLAB (REQUIRED FOR EACH LEVEL)
    - C.I.P. CONCRETE CMU WALL & COLUMNS (LAYOUT AND REINFORCING)
    - TIMBER FLOOR FRAMING AND SHEATHING (REQUIRED FOR EACH LEVEL)
    - TIMBER WALL AND ROOF FRAMING AND SHEATHING
    - FRAMING (BEAMS, COLUMNS, TRUSSES, STRIPS, HOLD DOWNS, SHEAR WALLS, ETC.)
  - FRAMING/STRAPPING (BEAMS, COLUMNS, TRUSSES, STRAPS, HOLD DOWNS, SHEAR WALLS, ETC.)
  - SITE IMPROVEMENTS:
    - SITE WALL LAYOUT AND REINFORCING
    - STORMWATER SITE GRADING
  - ENGINEER CERTIFICATION

## WALTON COUNTY FLOOD CERTIFICATION:

I CERTIFY, TO THE BEST OF MY KNOWLEDGE, THAT THE FOUNDATION AND STRUCTURE IS DESIGNED AS ADEQUATELY ANCHORED TO PREVENT FLOTATION, COLLAPSE AND LATERAL MOVEMENT OF THE STRUCTURE RESULTING FROM HYDRODYNAMIC AND HYDROSTATIC LOADS, INCLUDING THE EFFECT OF BUOYANCY. THE DESIGN HAS TAKEN INTO ACCOUNT THE PROVISIONS OF FBC 9222/FBC 1612/ASCE 241 AND THE FLOOD LOADS IMPOSED BY A BASE FLOOD EVENT OF A 100 YEAR FLOOD AS SHOWN ON THE CURRENT WALTON COUNTY FLOOD INSURANCE RATE MAP.

## WALTON COUNTY CERTIFICATION:

I CERTIFY THAT THE DESIGN PLANS AND SPECIFICATIONS FOR THIS CONSTRUCTION ARE IN COMPLIANCE WITH THE CRITERIA ESTABLISHED BY THE 2020 FLORIDA BUILDING CODE. THIS BUILDING AND/OR STRUCTURE IS DESIGNED TO WITHSTAND ULTIMATE WIND VELOCITY OF 140 MPH. ALSO, UPON COMPLETION OF THIS BUILDING AND/OR STRUCTURE, I WILL CERTIFY AT THAT TIME THE BUILDING AND/OR STRUCTURE HAS COMPLIED WITH THIS SPECIFIC BUILDING DESIGN. THIS MUST BE ON FILE AT THE WALTON COUNTY BUILDING DEPARTMENT BEFORE RECEIVING AN INSPECTION FOR POWER. I UNDERSTAND THAT ANY CHANGE IN DESIGN OR SPECIFICATION MUST BE SUBMITTED IN WRITING BY ME TO THE BUILDING DEPARTMENT. ALL DRAWINGS AND/OR CORRESPONDENCE SHALL BE SIGNED AND SEALED.

## PLAN SYMBOL LEGEND

	CONC. FOOTING
	THICKENED SLAB ON GRADE
	INDICATES LOCATION OF FOOTING STEP. VERIFY EXACT LOCATION IN THE FIELD
	INDICATES LOCATION OF FOOTING STEP. SEE ARCH. PLANS FOR DETAILS
	SOLID GROUTED CMU WALL/ PIER
	LOAD BEARING WOOD STUD WALL
	C.I.P. CONC. COLUMN / WALL
	LOAD BEARING POST/ COLUMN
	INDICATES LOCATION OF INTERIOR SHEARWALL SHEATHING. SEE SHEATHING SCHEDULE FOR NAIL SIZE & SPACING
	INDICATES LOCATION OF HOLD-DOWN. SEE HOLD-DOWN DETAILS & SCHEDULE FOR TYPE OF HARDWARE & ANCHOR
	INDICATES LOCATION OF WALL HEADER/ DROPPED BEAM

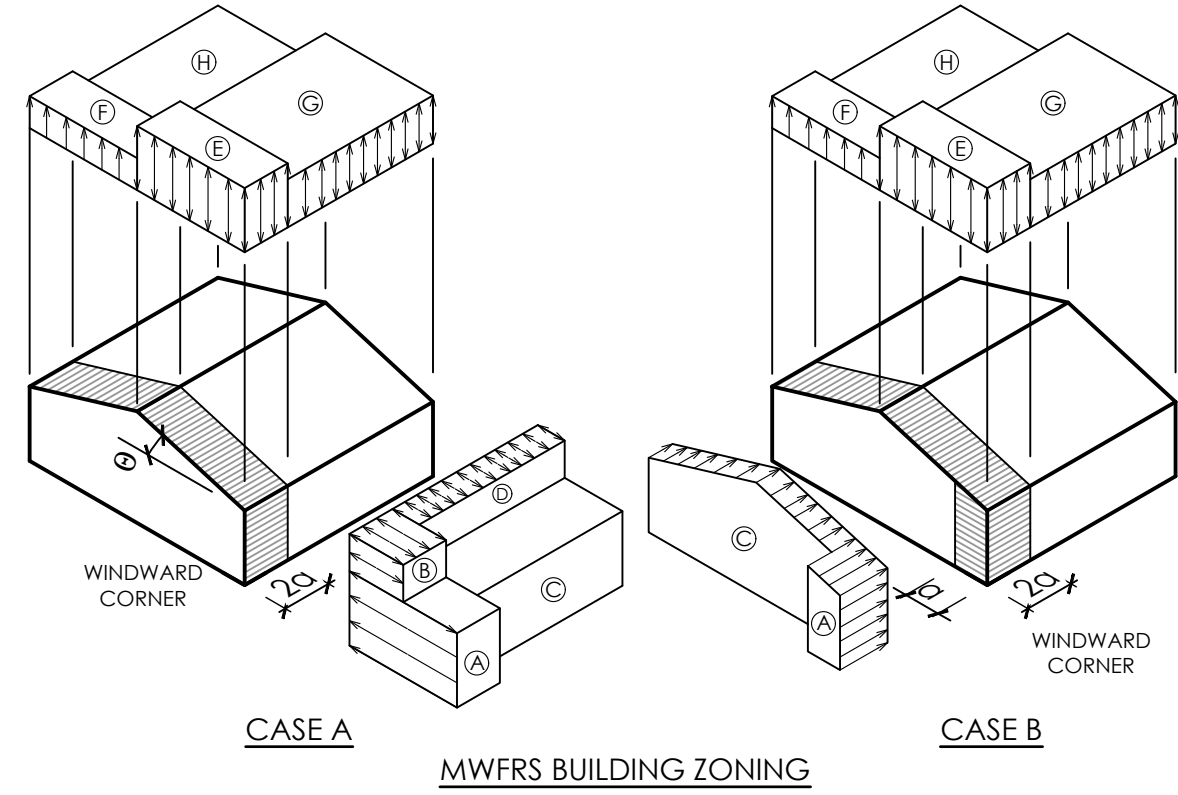
## STRUCTURAL MEMBER CALLOUT LEGEND

**WIND LOADING NOTES**

III	BUILDING RISK (OCCUPANCY) CATEGORY
160	ASCE 7-16 ULTIMATE WIND SPEED, $V_{ult}$ (MPH)
C	EXPOSURE CATEGORY
ENCLOSED	WIND CLOSURE CATEGORY
±0.18	INTERNAL PRESSURE COEFFICIENT, $C_{pi}$
10.0	END ZONE WIDTH, $a$ (FT)
30	MEAN ROOF HEIGHT, $h$ (FT)
1.40	ADJUSTMENT FACTOR FOR BUILDING HEIGHT AND EXPOSURE (FROM FIGURE 30.5-1 ASCE 7-16)

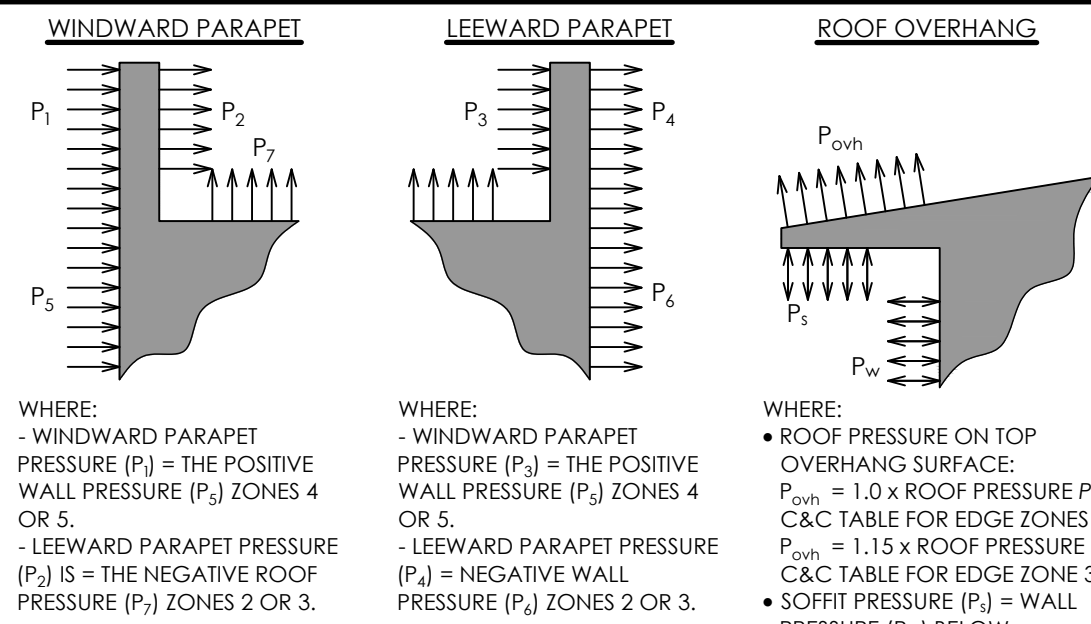
**ULTIMATE WIND PRESSURES FOR MAIN FRAME WIND RESISTING SYSTEM, PSF**

ROOF ANGLE	HORIZONTAL PRESSURES				VERTICAL PRESSURES				OVERHANGS	
	A	B	C	D	E	F	G	H	$E_{OH}$	$G_{OH}$
0 TO 5°	56.9	-29.4	37.7	-17.6		-38.8		-30.0		
10°	64.2	-26.5	42.6	-15.5		-41.7		-32.0		
15°	71.3	-23.6	47.5	-13.5	-68.2	-44.6	-47.5	-34.0	-95.6	-74.8
20°	78.6	-20.8	52.5	-11.5		-47.5		-36.0		
25°	71.3	11.5	51.6	11.7	-31.6	-43.2	-22.9	-34.7	-59.1	-50.3
30 TO 45°	64.0	43.7	50.8	34.9	4.9	-38.8	1.6	-33.3	-22.5	-25.6



**NOTE:**

- FOR CASE B USE  $\theta=0^\circ$ .
- PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE PROJECTED SURFACES, RESPECTIVELY.
- FOR ROOF SLOPES OTHER THAN THOSE SHOWN, LINEAR INTERPOLATION IS PERMITTED.
- WHERE ZONE E OR G FALLS ON A ROOF OVERHANG ON THE WINDWARD SIDE OF THE BUILDING, USE  $E_{OH}$  AND  $G_{OH}$  FOR THE PRESSURES ON THE HORIZONTAL PROJECTION OF THE OVERHANG. OVERHANGS ON THE LEeward SIDE EDGES SHALL HAVE THE BASIC ZONE PRESSURES APPLIED.
- PARAPET MWFRS PRESSURE = WALL PRESSURE TIMES 2.25 (PER FIGURE 27.5-2 ASCE 7-16).
- NEGATIVE PRESSURES IN ZONE B & D ARE GIVEN FOR REFERENCE AND INTERPOLATION PURPOSES ONLY AND SHALL BE IGNORED FOR DESIGNING MWFRS OF THE STRUCTURE.



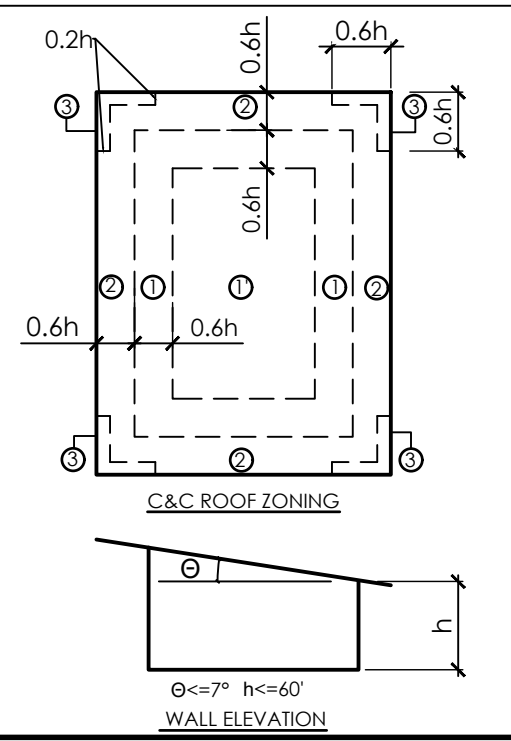
**NOTE:**

- C&C WIND LOADING IS BASED ON ASCE7-16 MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES.
- PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE PROJECTED SURFACES, RESPECTIVELY.
- ULTIMATE DESIGN WIND PRESSURE SHALL NOT BE LESS THAN 16 PSF IN ANY ZONE FOR ANY DIRECTION.
- FOR EFFECTIVE AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED. OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
- WHERE TESTING OF COMPONENTS AND CLADDING FOR WIND LOAD RESISTANCE IS BASED ON ALLOWABLE (NOMINAL) WIND LOADS (EX.: WINDOWS & DOORS), THE DESIGN WIND PRESSURES LISTED ABOVE SHALL BE MULTIPLIED BY 0.6.
- IF A PARAPET HEIGHT  $h_p \geq 3$  FT AROUND THE BUILDING PERIMETER WITH THE ROOF SLOPE  $\theta \leq 7^\circ$ , THE NEGATIVE VALUES FOR ZONE 3 = THE NEGATIVE VALUES FOR ZONE 2, AND POSITIVE VALUES IN ZONES 2 AND 3 SHALL BE SET EQUAL TO THOSE FOR WALL ZONES 4 AND 5, RESPECTIVELY.

**FLAT/HIP/GABLE ROOF ULTIMATE WIND PRESSURES FOR COMPONENTS & CLADDING, PSF**

**ROOF SLOPE FROM  $\frac{1}{12}$  TO  $\frac{1.47}{12}$  ( $0^\circ \leq \theta \leq 7^\circ$ )**

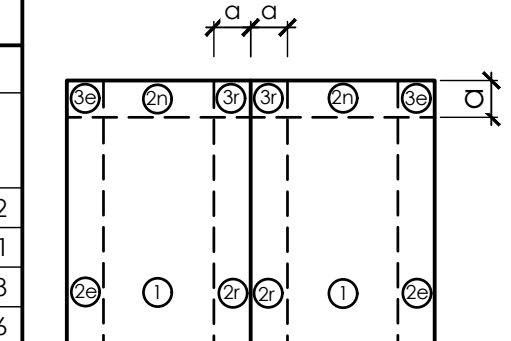
AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE				OVERHANG NET PRESSURE		
		ZONE 1	ZONE 1'	ZONE 2	ZONE 3	ZONE 1, 1'	ZONE 2	ZONE 3
≤10	26.1	-102.8	-59.1	-135.5	-184.7	-92.9	-125.6	-174.8
20	24.5	-96.0	-59.1	-126.7	-167.3	-91.2	-114.1	-154.5
50	22.5	-87.0	-59.1	-115.2	-144.3	-89.1	-98.7	-127.6
100	20.8	-80.3	-59.1	-106.6	-126.7	-87.4	-87.0	-107.3



**GABLE ROOF ULTIMATE WIND PRESSURES FOR COMPONENTS & CLADDING, PSF**

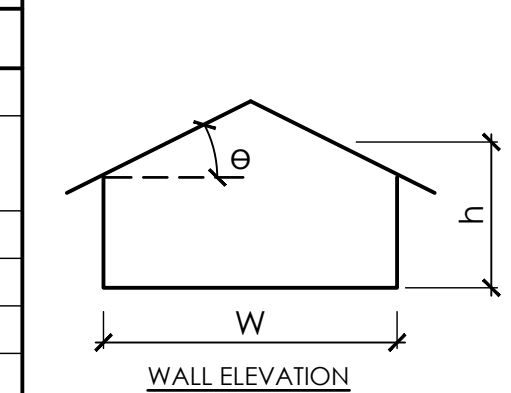
**ROOF SLOPE FROM  $\frac{1}{12}$  TO  $\frac{4.37}{12}$  ( $7^\circ < \theta \leq 20^\circ$ )**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE			OVERHANG NET PRESSURE		
		ZONE 1, 2e	ZONE 2a, 2b, 3e	ZONE 3r	ZONE 1, 2e	ZONE 2a, 2b	ZONE 3e
≤10	39.1	-119.0	-173.7	-206.4	-136.6	-191.3	-224.0
20	35.3	-119.0	-150.1	-177.0	-136.6	-166.6	-193.5
50	30.2	-72.4	-119.0	-137.9	-118.1	-134.0	-153.1
100	26.1	-37.1	-95.6	-108.3	-104.0	-109.3	-122.3



**ROOF SLOPE FROM  $\frac{4.37}{12}$  TO  $\frac{6.11}{12}$  ( $20^\circ < \theta \leq 27^\circ$ )**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE			OVERHANG NET PRESSURE		
		ZONE 1, 2e	ZONE 2a, 2b, 3e	ZONE 3r	ZONE 1, 2e	ZONE 2a, 2b	ZONE 3e
≤10	39.1	-91.8	-146.5	-188.7	-109.3	-163.8	-196.8
20	35.3	-91.8	-128.2	-154.0	-109.3	-152.7	-165.5
50	30.2	-77.9	-104.2	-108.3	-105.5	-137.9	-124.2
100	26.1	-67.5	-86.1	-108.3	-102.8	-126.7	-136.6



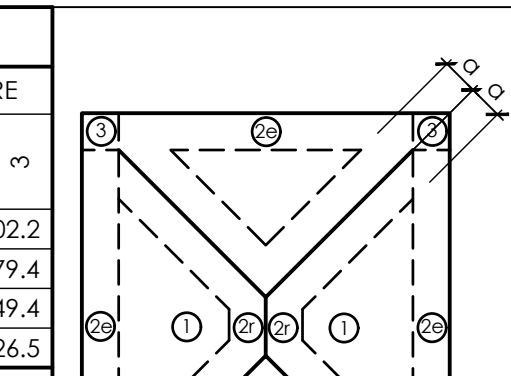
**ROOF SLOPE FROM  $\frac{6.11}{12}$  TO  $\frac{12}{12}$  ( $27^\circ < \theta \leq 45^\circ$ )**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE			OVERHANG NET PRESSURE		
		ZONE 1, 2e	ZONE 2a, 2b, 3e	ZONE 3r	ZONE 1, 2e	ZONE 2a, 2b	ZONE 3e
≤10	59.1	-108.3	-119.0	-146.1	-120.1	-153.1	-179.9
20	52.5	-91.8	-106.4	-129.5	-111.9	-140.4	-163.3
50	43.7	-70.0	-89.8	-107.5	-101.1	-123.6	-141.3
100	37.1	-53.6	-77.2	-90.9	-92.9	-111.0	-124.7

**HIP ROOF ULTIMATE WIND PRESSURES FOR COMPONENTS & CLADDING, PSF**

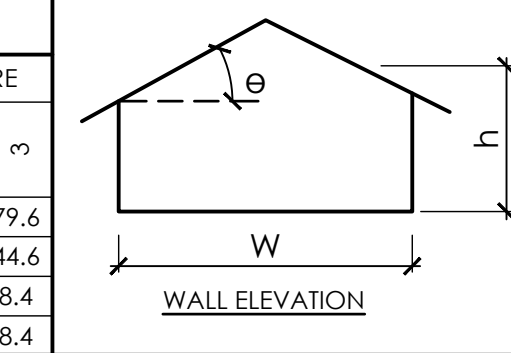
**ROOF SLOPE FROM  $\frac{1}{12}$  TO  $\frac{4.37}{12}$  ( $7^\circ < \theta \leq 20^\circ$ )**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE				OVERHANG NET PRESSURE		
		ZONE 1	ZONE 2e	ZONE 2r	ZONE 3	ZONE 1	ZONE 2e	ZONE 3
≤10	48.1	-108.3	-152.0	-141.0	-152.0	-125.6	-169.3	-202.2
20	41.5	-108.3	-136.8	-127.1	-136.8	-112.5	-160.5	-179.4
50	32.7	-83.4	-116.7	-108.6	-116.7	-95.1	-148.8	-149.4
100	26.1	-64.5	-101.5	-94.7	-101.5	-81.9	-140.1	-126.5



**ROOF SLOPE FROM  $\frac{4.37}{12}$  TO  $\frac{6.11}{12}$  ( $20^\circ < \theta \leq 27^\circ$ )**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE				OVERHANG NET PRESSURE		
		ZONE 1	ZONE 2e	ZONE 2r	ZONE 3	ZONE 1	ZONE 2e	ZONE 3
≤10	48.1	-86.3	-119.0	-119.0	-103.5	-133.7	-133.7	-160.0
20	41.5	-76.4	-106.4	-106.4	-106.4	-128.0	-128.0	-142.1
50	32.7	-63.5	-89.8	-89.8	-89.8	-100.8	-120.5	-118.1
100	26.1	-53.6	-77.2	-77.2	-77.2	-99.5	-114.8	-100.0



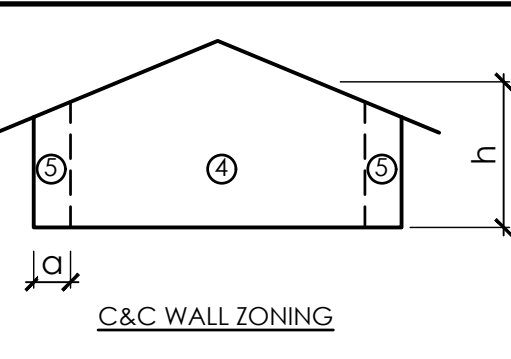
**ROOF SLOPE FROM  $\frac{6.11}{12}$  TO  $\frac{12}{12}$  ( $27^\circ < \theta \leq 45^\circ$ )**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE				OVERHANG NET PRESSURE		
		ZONE 1	ZONE 2e	ZONE 2r	ZONE 3	ZONE 1	ZONE 2e	ZONE 3
≤10	45.5	-91.8	-109.7	-149.2	-145.7	-125.6	-151.4	-183.0
20	39.7	-81.7	-86.9	-123.6	-110.8	-115.6	-125.4	-144.6
50	32.0	-68.4	-56.7	-90.0	-64.5	-102.2	-91.1	-123.8
100	26.1	-58.1	-53.6	-64.5	-64.5	-92.2	-87.4	-98.4

**ULTIMATE WIND PRESSURES FOR COMPONENTS & CLADDING, PSF**

**WALLS**

AREA, FT²	POSITIVE PRESSURE ALL ZONES	NEGATIVE PRESSURE	
		ZONE 4	ZONE 5
≤10	35.3	-38.2	-47.2
20	33.7	-36.7	-44.0
50	31.6	-34.6	-39.8
100	30.0	-33.0	-36.7



**DESIGN PRESSURE RATINGS FOR WINDOWS AND DOORS, PSF**

ZONE	OPENING AREA, FT²									
	≤ 10	20	30	40	50	60	70	80	90	100
ZONE 4.5, POSIT. PRESSURE	38.6	36.9	35.9	35.2	34.6	34.1	33.8	33.5	33.1	32.9
ZONE 4, NEGAT. PRESSURE	-41.9	-40.2	-39.2	-38.4	-37.9	-37.4	-37.1	-36.8	-36.4	-36.1
ZONE 5, NEGAT. PRESSURE	-51.8	-48.3	-46.2	-44.8	-43.7	-43.0	-42.0	-41.3	-40.7	-40.2

**NOTE:**

- PER 1709.5 EXTERIOR WINDOW AND DOOR ASSEMBLIES, FLORIDA BUILDING CODE 2020, THE DESIGN PRESSURE RATINGS OF EXTERIOR WINDOWS AND DOORS IN BUILDINGS SHOWN ABOVE ARE DETERMINED BY USING ASCE7 ULT. WIND PRESSURES MULTIPLIED BY 0.6.
- FOR OPENING AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED. OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
- SEE ARCHITECTURAL PLANS FOR WINDOW/DOOR TYPES, SIZES AND LOCATIONS.



**DAG ARCHITECTS**  
DAG Architects, AAC000745  
1223 Airport Road Destin, Florida 32541  
850.837.8152 www.dagarchitects.com

PRELIM SET

South Walton County Mosquito Control District  
**New Headquarters Building**

774 North County Highway 393  
Santa Rosa Beach, Florida 32459

PRELIMINARY  
NOT FOR CONSTRUCTION

No.	Description	Date

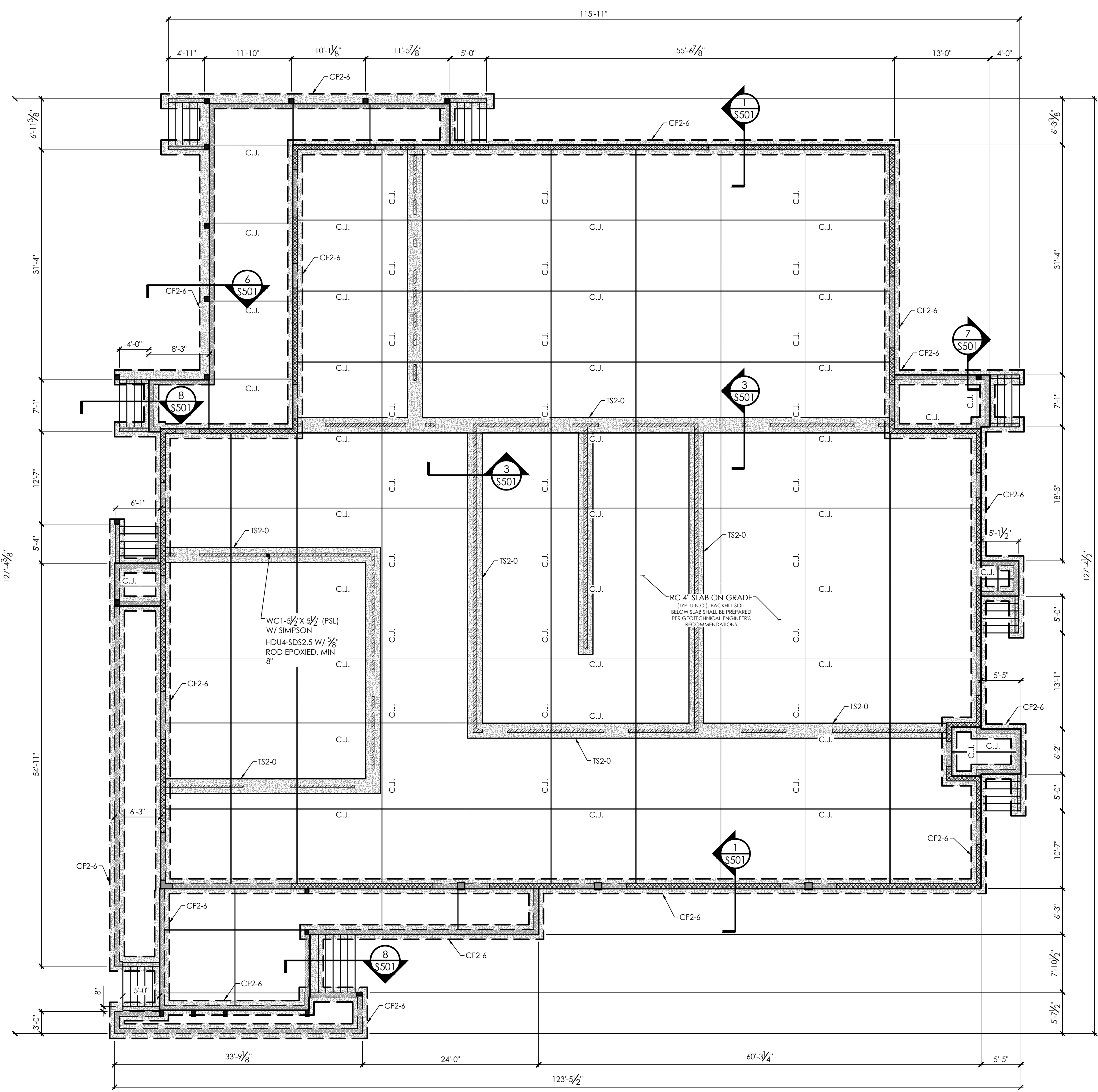
WIND LOAD TABLES

**O'CONNELL & ASSOCIATES**  
CONSULTING ENGINEERS, LLC.  
CERT. OF AUTH. #30549  
1394 Co. Hwy. 283 S, Bldg. #3 Santa Rosa Beach FL 32459  
(850) 403-4555 www.oconnellengineers.com

OA Project number: 18178  
DAG Project number: 18106  
Date: 08.02.23  
PIC: NOC  
PM: JMB

S002

ALL REPORTS, PLANS, SPECIFICATIONS, COMPUTERS FILES, RECORDS, NOTES AND OTHER DOCUMENTS AND INSTRUMENTS SHALL REMAIN THE PROPERTY OF THE CONSULTANT SHALL BE RETURNED TO THE CONSULTANT UPON COMPLETION OF THE PROJECT. THE CONSULTANT SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE INSTRUMENTS OF SERVICE. THE CONSULTANT SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE INSTRUMENTS OF SERVICE. THE CONSULTANT SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE INSTRUMENTS OF SERVICE.



FOUNDATION SCHEDULE								
MARK	TYPE	SIZE			BOTTOM REINFORCING BARS		TOP REINFORCING BARS	
		WIDTH W, FT	LENGTH L, FT	THICKNESS T, FT	B <sub>w</sub>	B <sub>l</sub>	T <sub>w</sub>	T <sub>l</sub>
CF2-6	CONT. FOOTING	2'-6"	CONT.	1'-0"	#3 X 2'-0" @ 24" O/C	3-#5 CONT.	NONE	
TS2-0	THICKENED SLAB	2'-0"	CONT.	1'-0"	#3 X 1'-6" @ 24" O/C	3-#5 CONT.	NONE	

**NOTE:**  
 1. SEE 1-4/S400 FOR TYP. FOOTING DETAILS.  
 2. SEE 7/S400 FOR TYP. SLAB-ON-GRADE DETAILS.  
 3. TOP OF FOOTING SHALL BE PLACED **MIN. 1'-0"** BELOW FINISH GRADE.  
 4. REINFORCING BARS SHALL HAVE THE FOLLOWING CONCRETE COVER:  
 - BOTTOM & SIDE COVER = 3"  
 - TOP COVER = 2"  
 REINFORCING BAR PLACEMENT & COVER TOLERANCE = ± 1/2".  
 5. CONCRETE SHALL HAVE MIN. 3000 PSI COMPRESSIVE STRENGTH.

- FOUNDATION PLAN NOTES:**
- FOR GENERAL NOTES AND DESIGN LIVE LOADS SEE SHEET S001, FOR DETAILS SEE S500 SERIES SHEETS.
  - ALL FOOTING TRENCHES SHOULD BE CLEARED OF ORGANIC MATERIALS.
  - COORDINATE PROPERTY LINE FOOTING DEPTHS WITH ALL EXISTING ADJACENT PROPERTY LINE FOOTING DEPTHS PRIOR TO START OF CONSTRUCTION.
  - COORDINATE MEP ROUGH INS PRIOR TO CONSTRUCTION
  - BOTTOM OF FOOTING TO BE PLACED MINIMUM 1'-6" BELOW FINISHED GRADE.
  - SEE ARCH. DRAWINGS FOR SLOPES, DRAINS, OPENINGS, FLOOR RECESSES AND DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS. (SEE CONTRACTOR NOTE BELOW, FOR EXCEPTION).
  - CMU WALLS ARE MIN. 8" (NOM.) THICK. FREE STANDING CMU PIERS ARE MIN. 16" (NOM.) SQ. SEE TYPICAL DETAIL FOR REINF. REQS.
  - CONTRACTOR SHALL COORDINATE FOOTING STEP LOCATION AND DEPTH IN THE FIELD BASED ON FINISH GRADE ELEVATIONS. SEE TYP. FOOTING STEP DETAIL FOR REINF. LAYOUT & DIMENSIONS

**1** FOUNDATION/SLAB ON GRADE PLAN  
 SCALE: 1/8"=1'-0"

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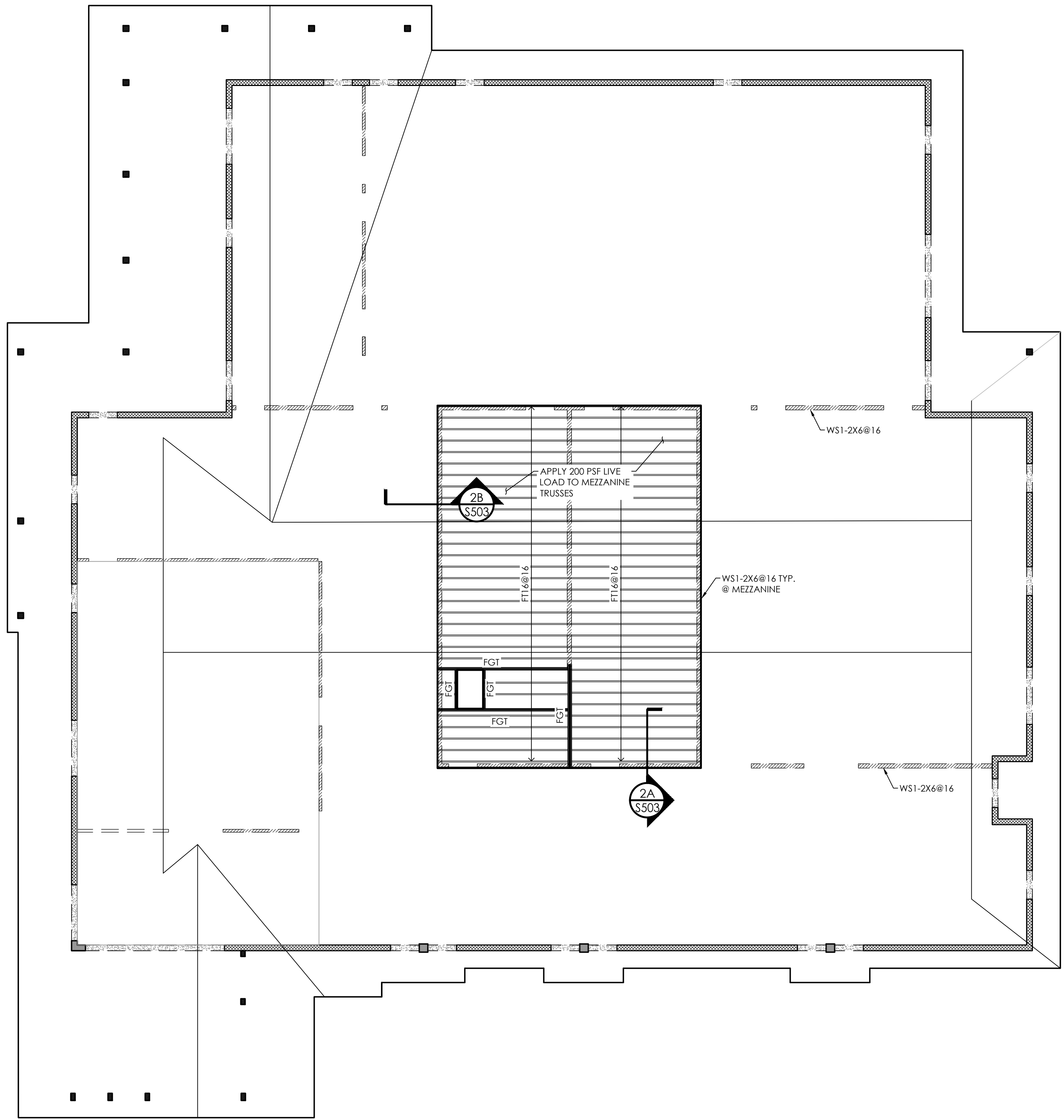
No.	Description	Date

FOUNDATION PLAN

OA Project number:	18178
DAG Project number:	18106
Date:	08.02.23
PIC:	NOC
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S101

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**PLAN NOTES:**

1. FOR GENERAL NOTES AND DESIGN LIVE LOADS SEE SHEET S001.
2. FOR DETAILS SEE S500 SERIES SHEETS.
3. SEE ARCH. DRAWINGS FOR SLOPES, DRAINS, OPENINGS, FLOOR RECESSES, DIMENSIONS, AND ELEVATION HEIGHTS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS.
4. COORDINATE MEP ROUGH INS PRIOR TO CONSTRUCTION
5. PRE-ENGINEERED TRUSS SHOP DRAWINGS MUST BE REVIEWED AND APPROVED BY ARCHITECT, EOR, & CONTRACTOR PRIOR TO FABRICATION AND INSTALLATION.
6. TRUSS TO TRUSS CONNECTORS AND TRUSS BRIDGING SHALL BE SPECIFIED BY TRUSS MFR.
7. CONTRACTOR IS RESPONSIBLE TO COORDINATE EXACT TRUSS LOCATION, REQUIRED OPENINGS & RECESSES WITH ARCH. & MEP PLANS.
8. STAIRS AND HADRAILS SHALL BE DESIGNED BY A DELEGATED ENGINEER. SUBMIT SHOP DRAWINGS TO ARCHITECT AND EOR FOR APPROVAL.



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**MEZZANINE FRAMING PLAN**

OA Project number:	18178
DAG Project number:	18106
Date:	08.02.23
PIC:	NOC
PM:	JMB

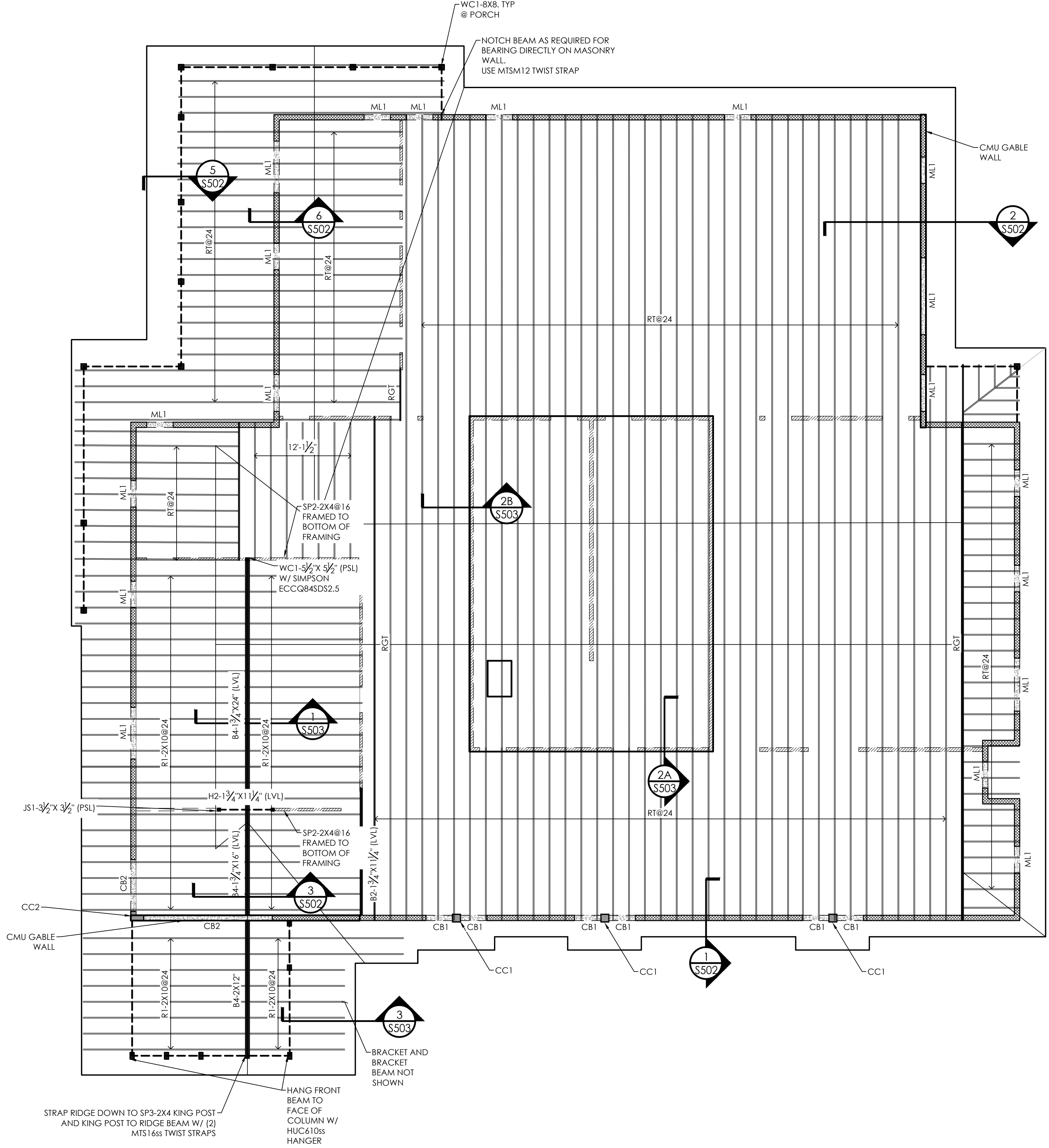
S102

1 MEZZANINE FRAMING PLAN  
 SCALE: 1/8"=1'-0"

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- MAIN ROOF FRAMING PLAN NOTES:**
- FOR GENERAL NOTES AND DESIGN LIVE LOADS SEE SHEET S001. FOR TYPICAL DETAILS SEE S500 SERIES SHEETS.
  - SEE ARCH. DRAWINGS FOR ROOF & CEILING SLOPES, ELEVATION HEIGHTS, AND DIMENSIONS NOT SHOWN. IF A CONFLICT EXISTS, THE ARCH. DIMENSIONS SHALL GOVERN. BOTH ENGINEER AND ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING OF ALL CONFLICTS.
  - PRE-ENGINEERED TRUSS SHOP DRAWINGS MUST BE REVIEWED AND APPROVED BY ARCHITECT, EOR, & CONTRACTOR PRIOR TO FABRICATION AND INSTALLATION.
  - TRUSS TO TRUSS CONNECTORS, CONNECTORS TO CMU AND TO CONCRETE WALLS, AND TRUSS BRIDGING SHALL BE SPECIFIED BY TRUSS MFR.
  - CONTRACTOR IS RESPONSIBLE TO COORDINATE EXACT TRUSS LOCATION, REQUIRED OPENINGS & RECESSES WITH ARCH. & MEP PLANS.



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ROOF FRAMING PLAN

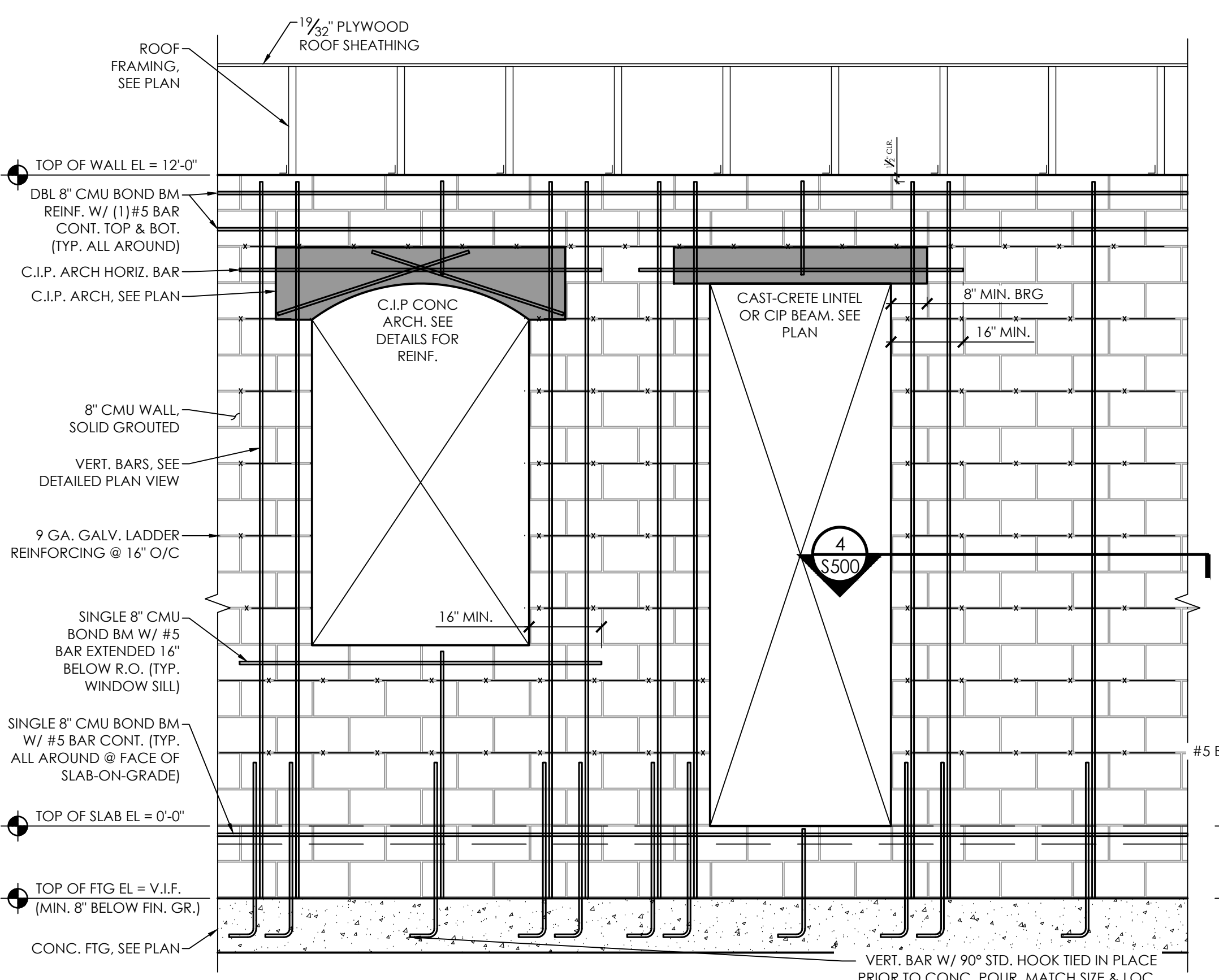
1 ROOF FRAMING PLAN  
SCALE: 1/8"=1'-0"

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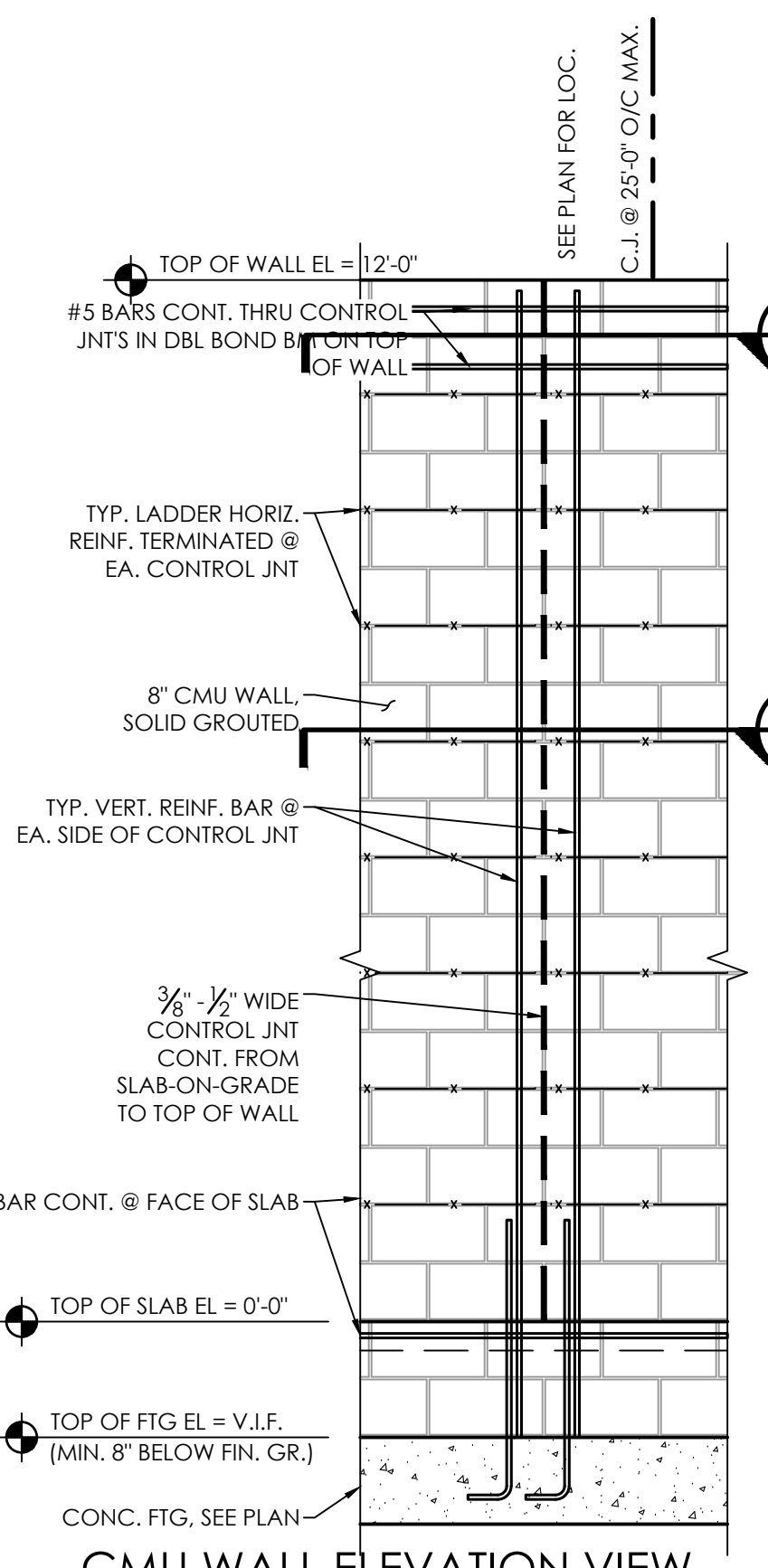
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S103

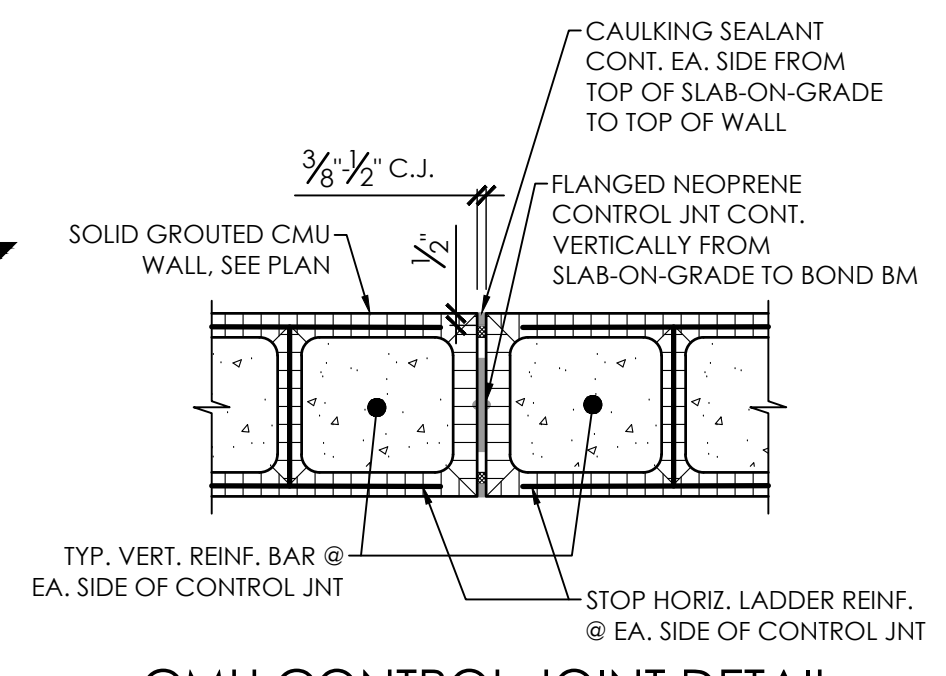
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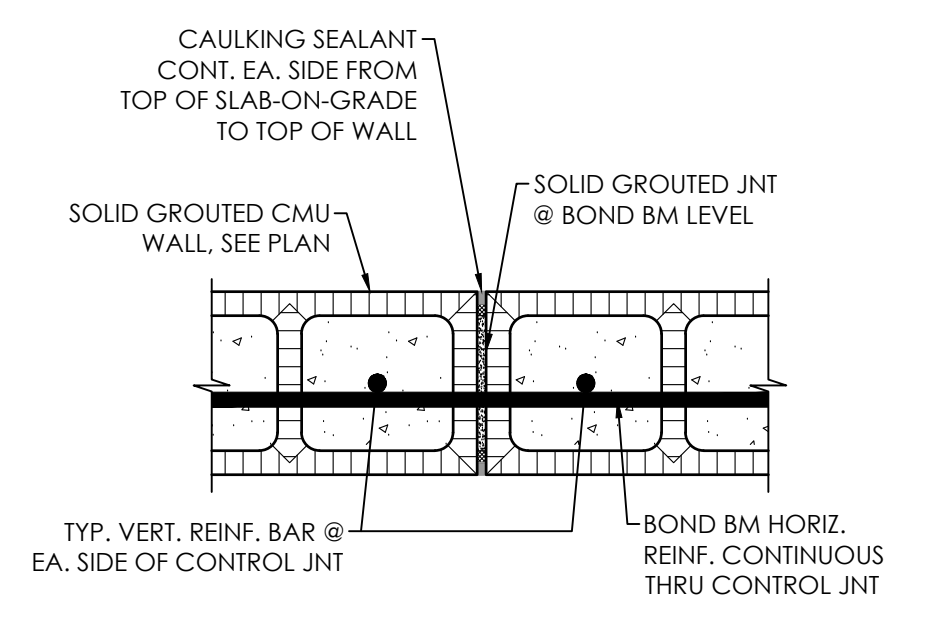
1 TYP. CMU WALL REINFORCING DETAIL. ELEVATION VIEW  
SCALE: 1/2" = 1'-0"



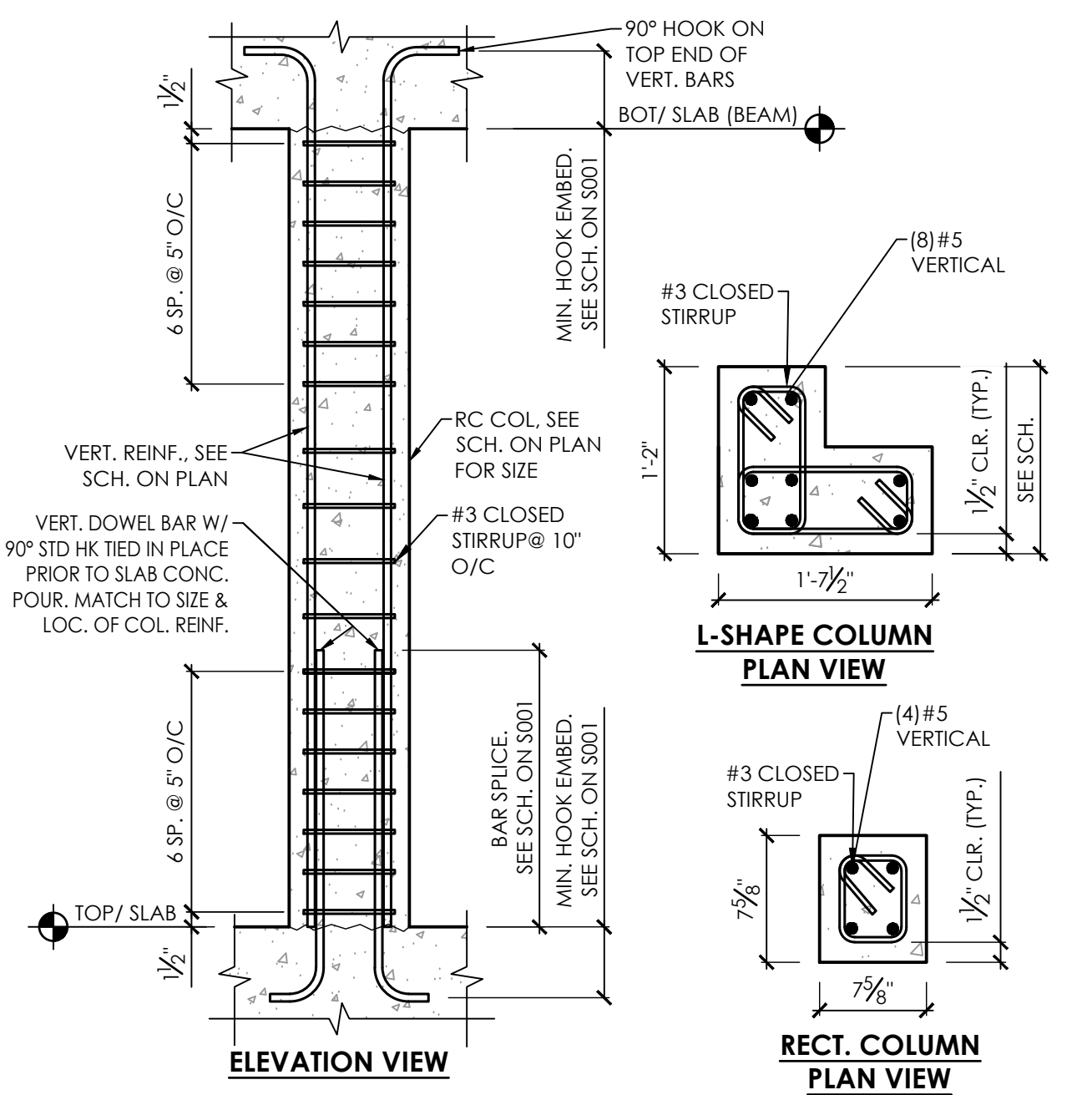
2 CMU WALL ELEVATION VIEW @ CONTROL JOINT LOC.  
SCALE: 1/2" = 1'-0"



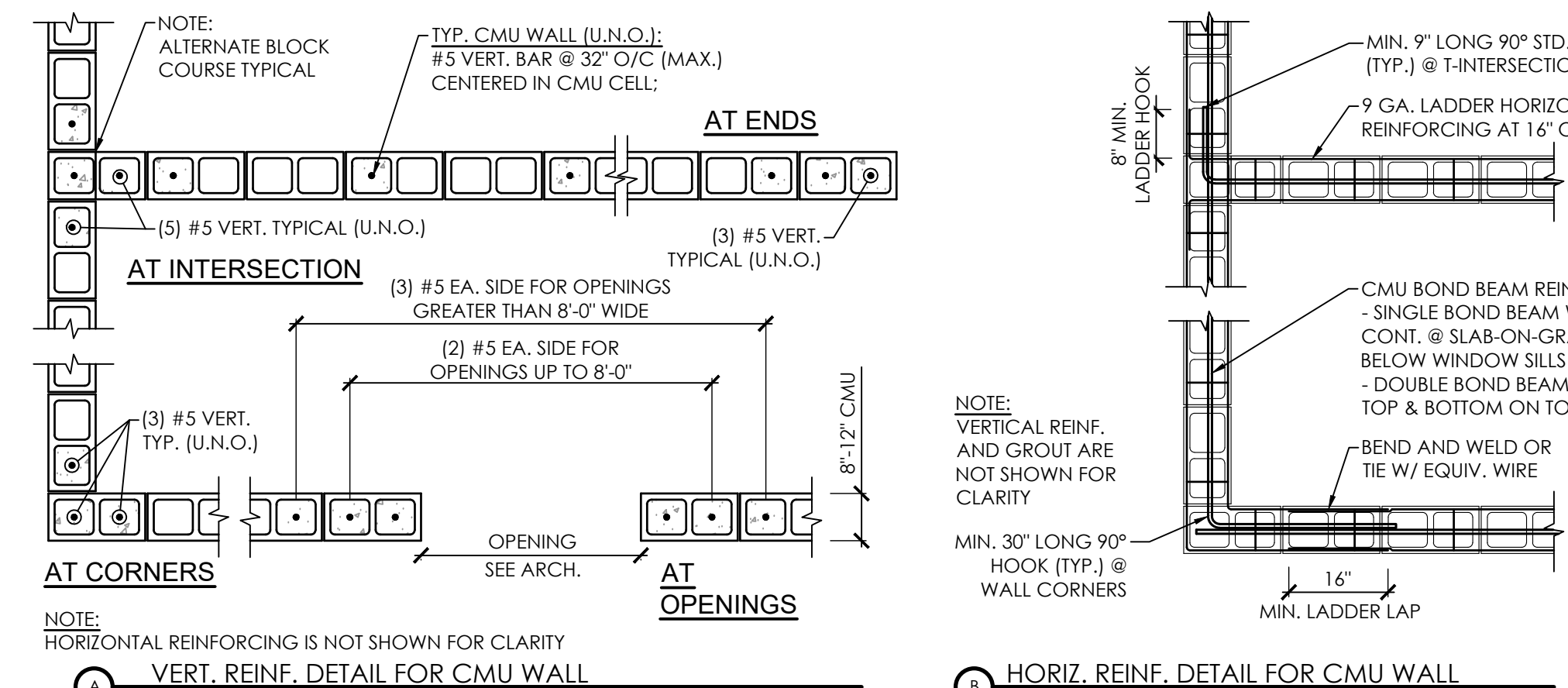
CMU CONTROL JOINT DETAIL @ LADDER REINFORCING  
SCALE: 1-1/2" = 1'-0"



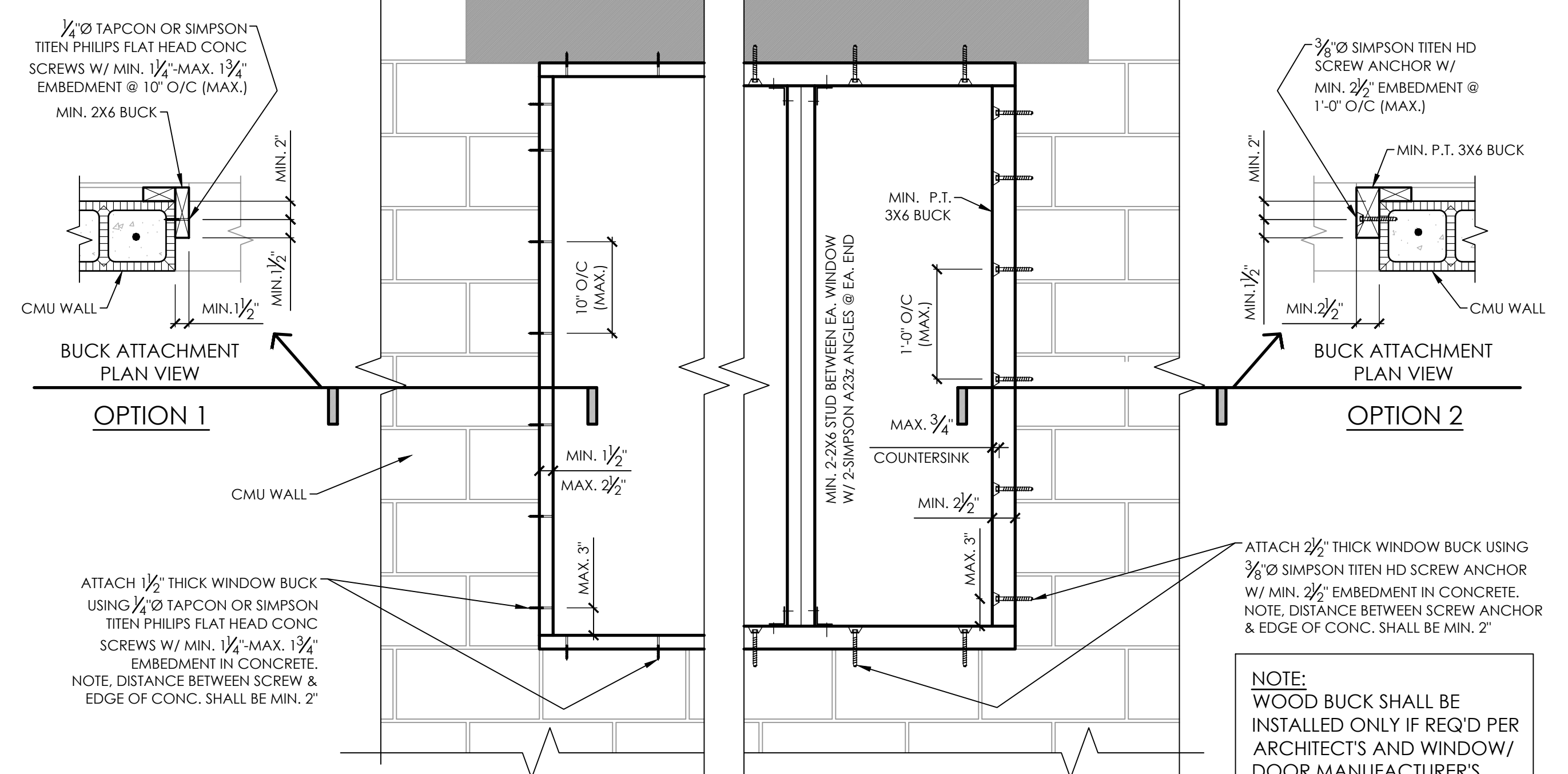
CMU WALL ELEVATION VIEW @ JOINT LOC.  
SCALE: 1/2" = 1'-0"



3 TYP. CONCRETE COLUMN REINF. DETAIL  
SCALE: N.T.S.



4 TYP. CMU WALL REINFORCING DETAILS. PLAN VIEW  
SCALE: N.T.S.



5 TYP. WINDOW/ DOOR BUCK CONNECTION DETAIL  
SCALE: 1" = 1'-0"



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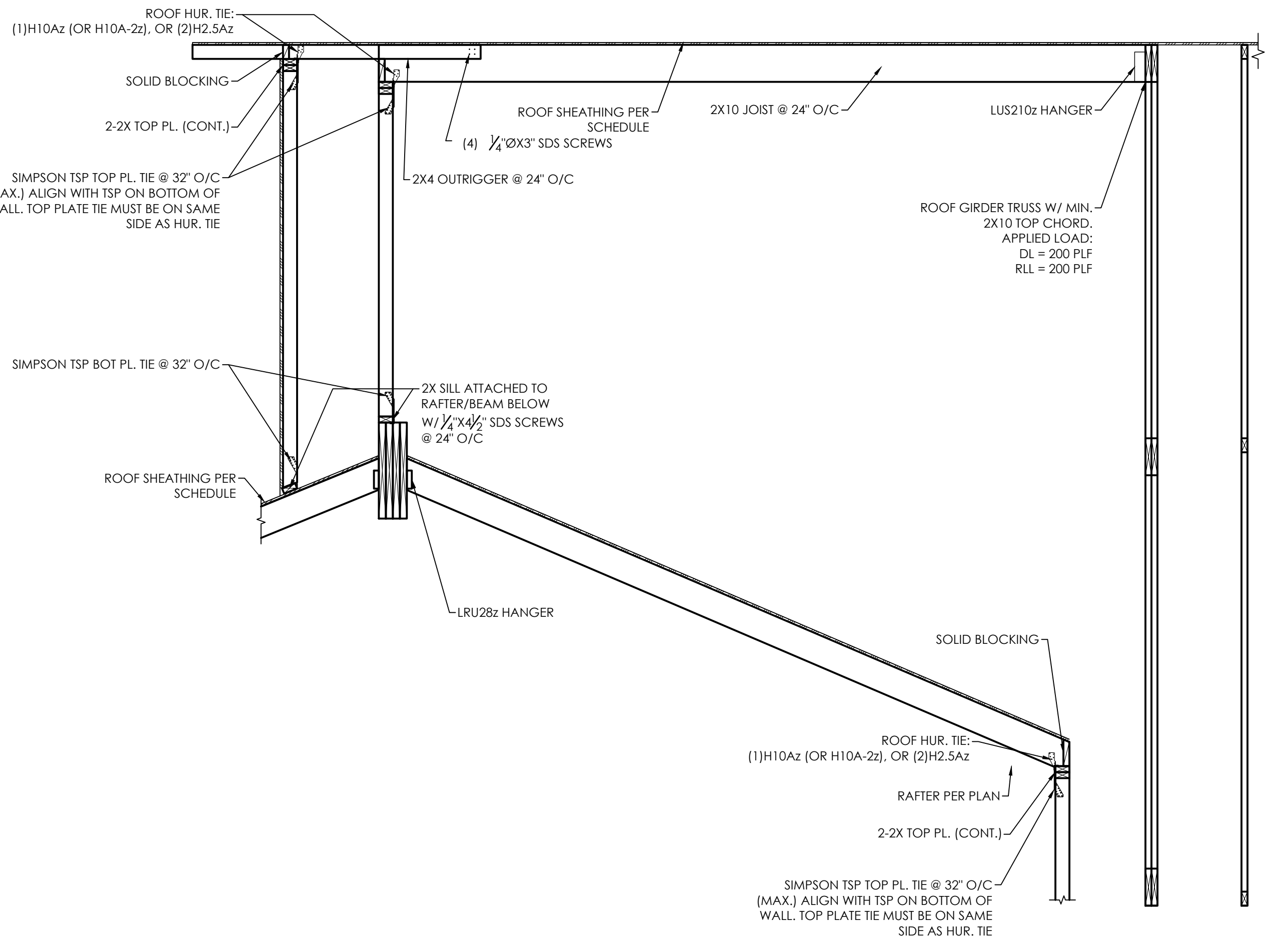
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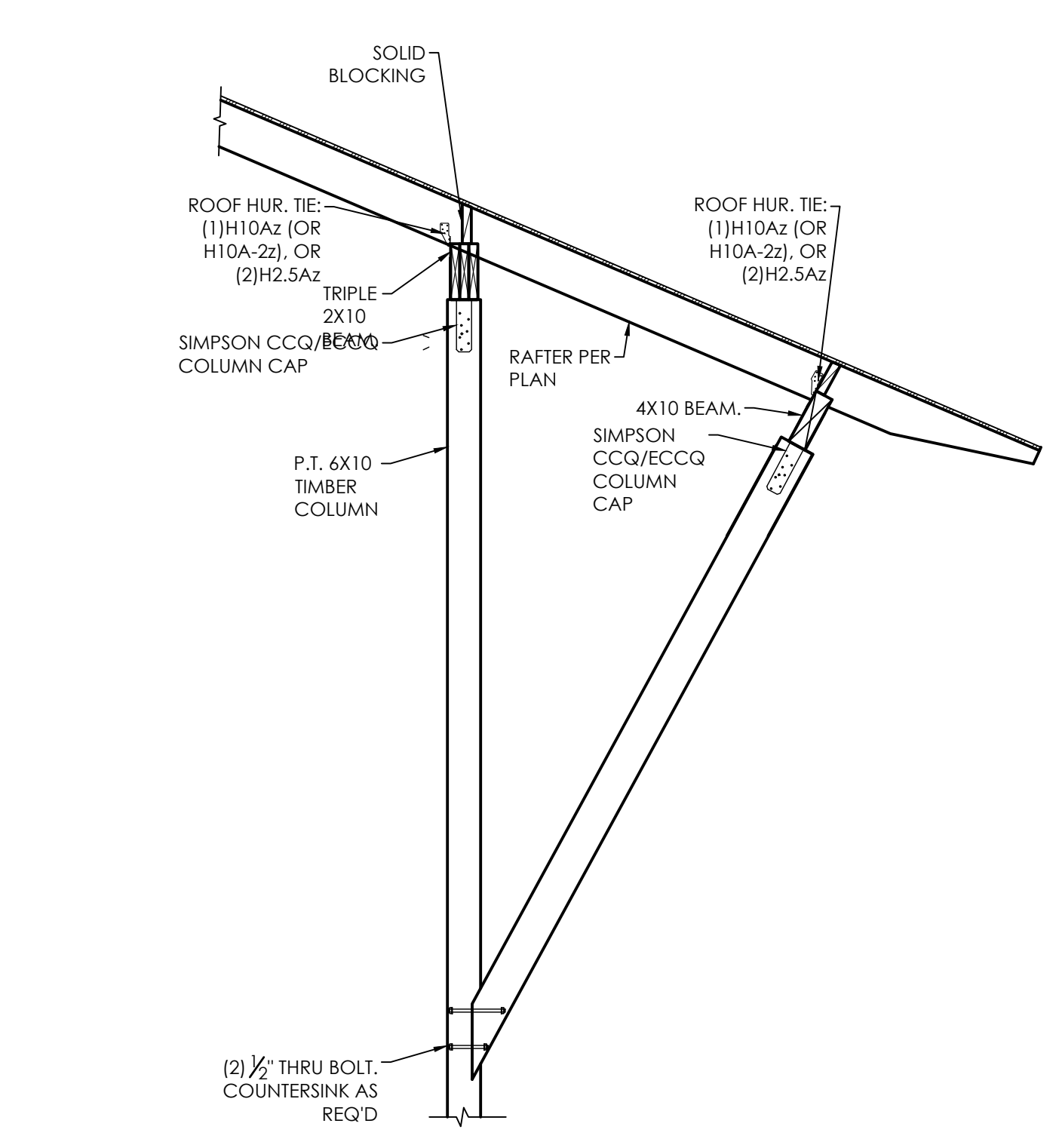




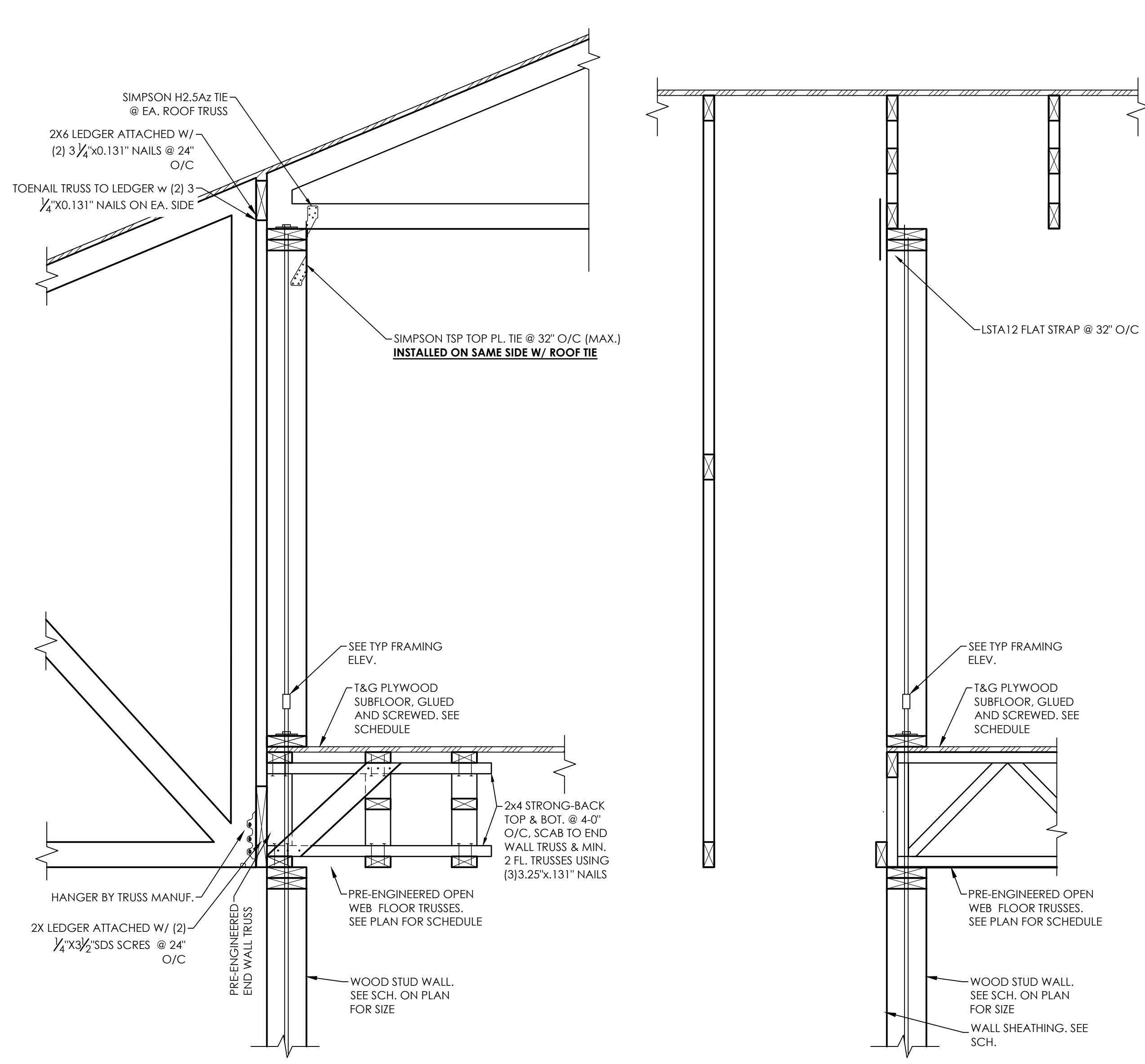
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**1 VAULTED ROOM FRAMING DETAIL**  
 S503 SCALE: 1"=1'-0"



**3 ENTRY BRACKET DETAIL**  
 S503 SCALE: 1"=1'-0"



**A TRUSS BEARING ON WALL**

**2 ELEVATED FLOOR FRAMING DETAIL @ STUD WALL**  
 S503 SCALE: 1"=1'-0"

**A TRUSS BEARING ON WALL**



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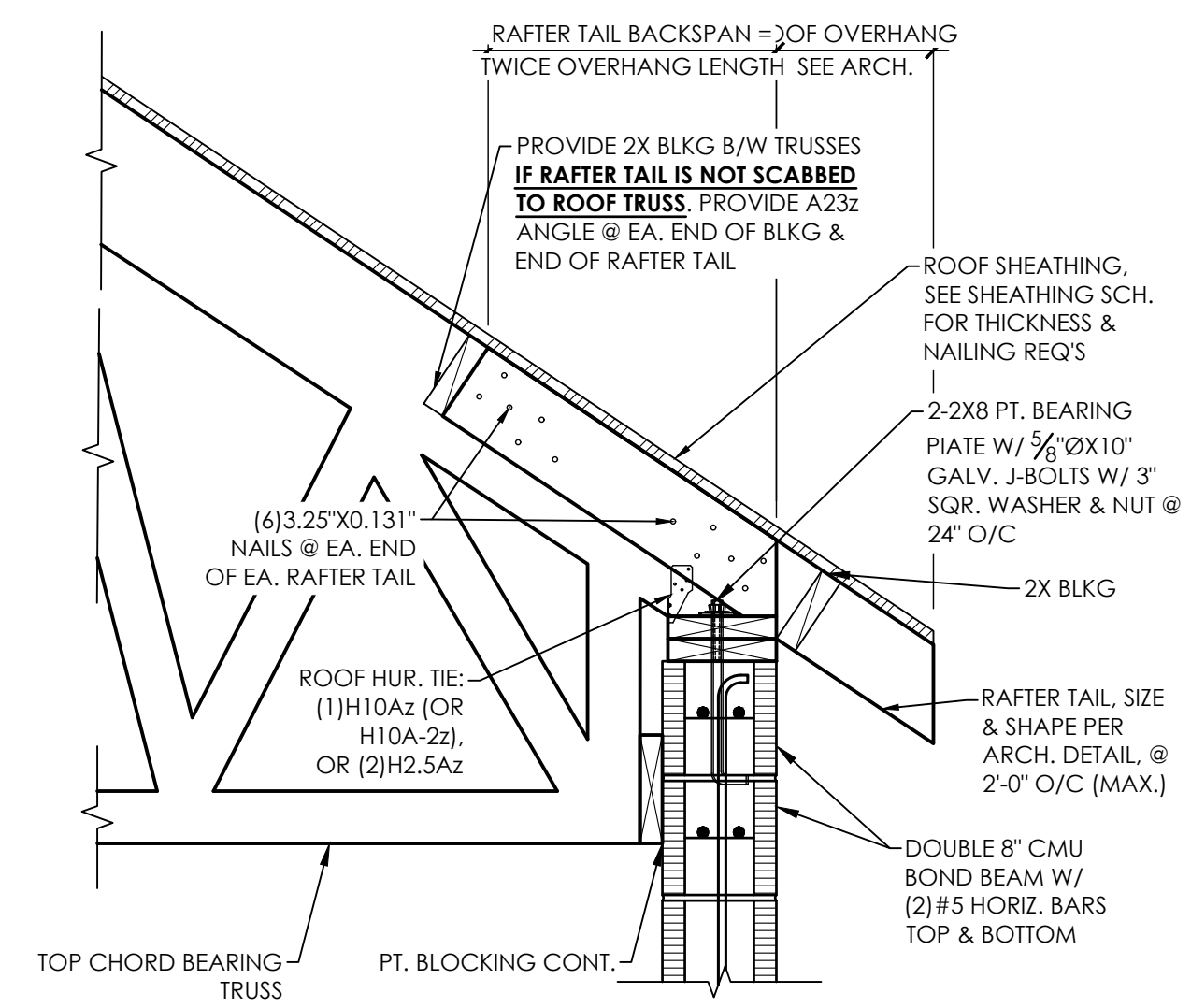
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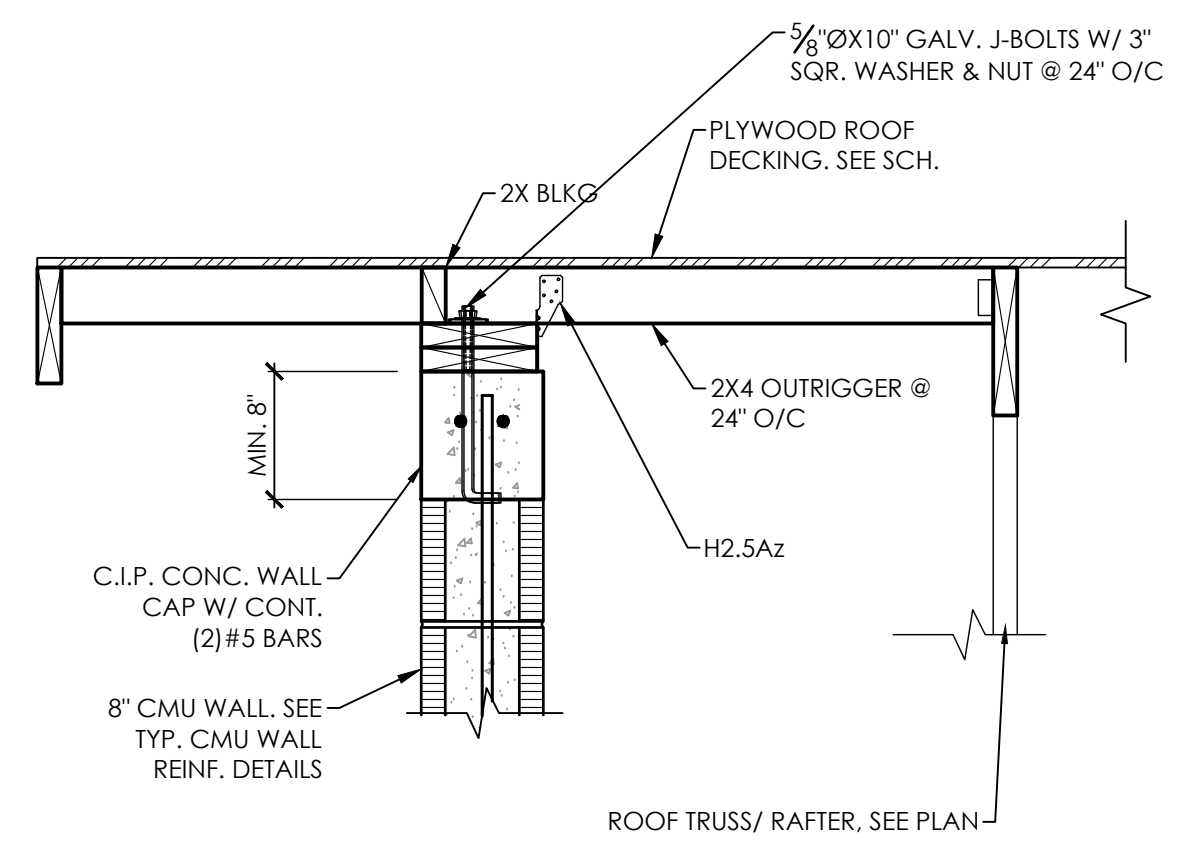
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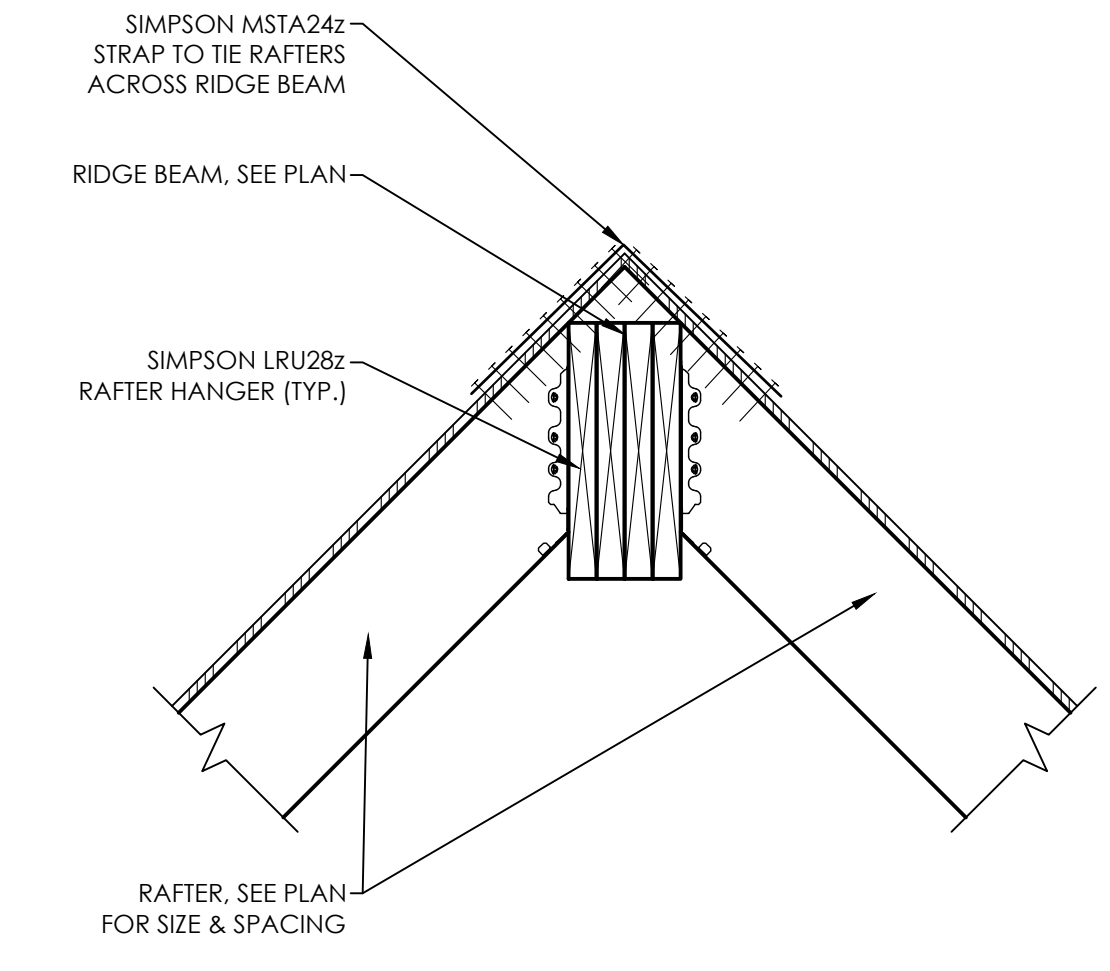
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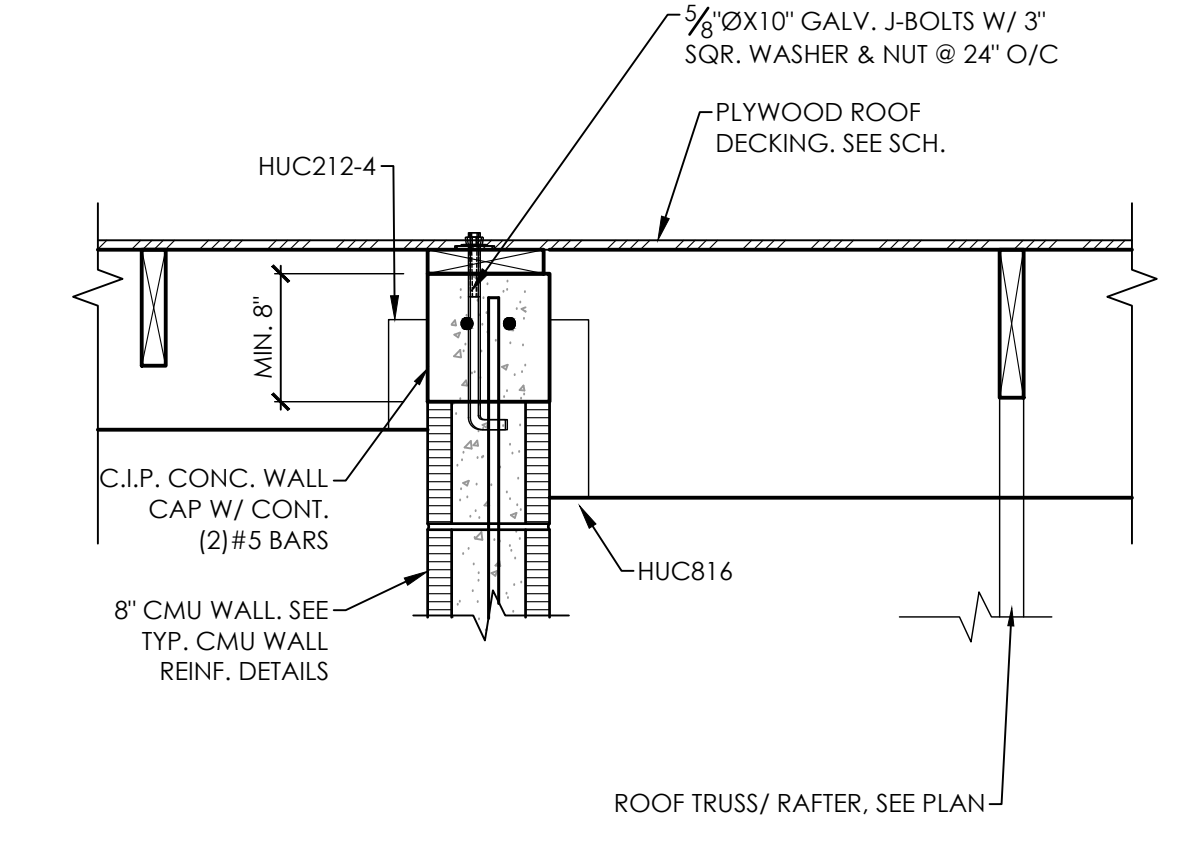
**1 TYP. ROOF TRUSS DETAIL**  
 SCALE: 1"=1'-0"



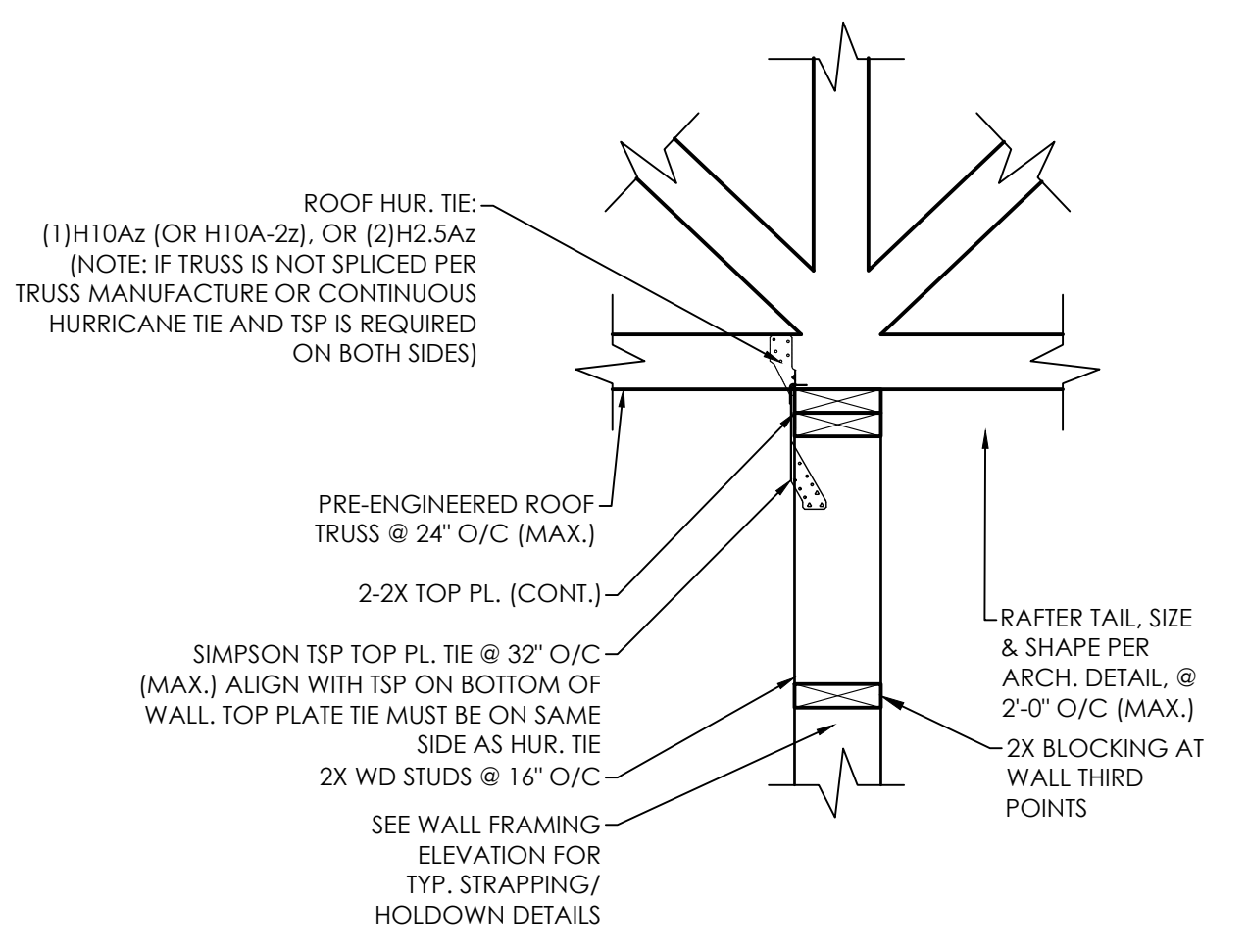
**2 TYP. GABLE END DETAIL**  
 SCALE: 1"=1'-0"



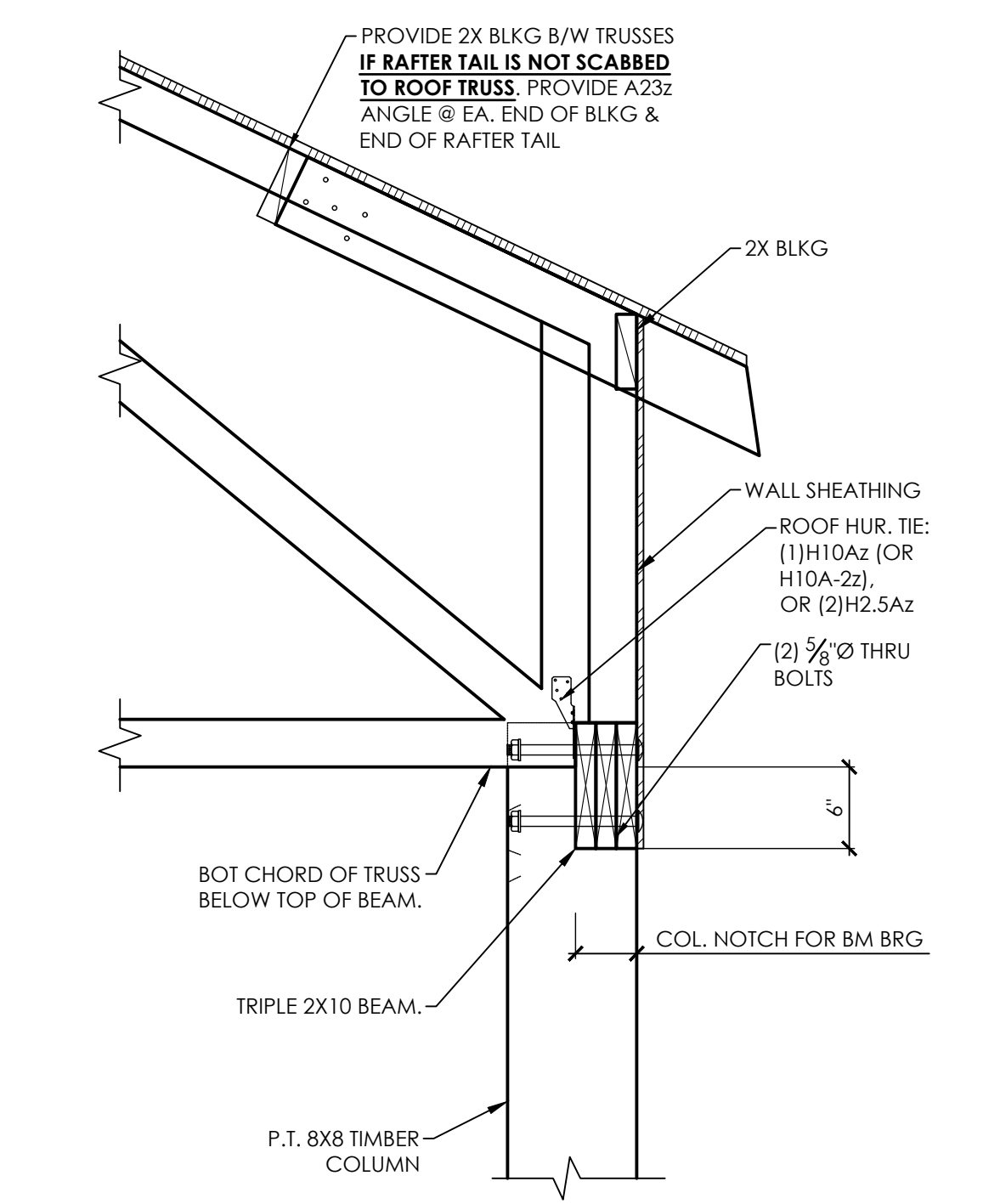
**3 ROOF TRUSS INT. BEARING**  
 SCALE: 1"=1'-0"



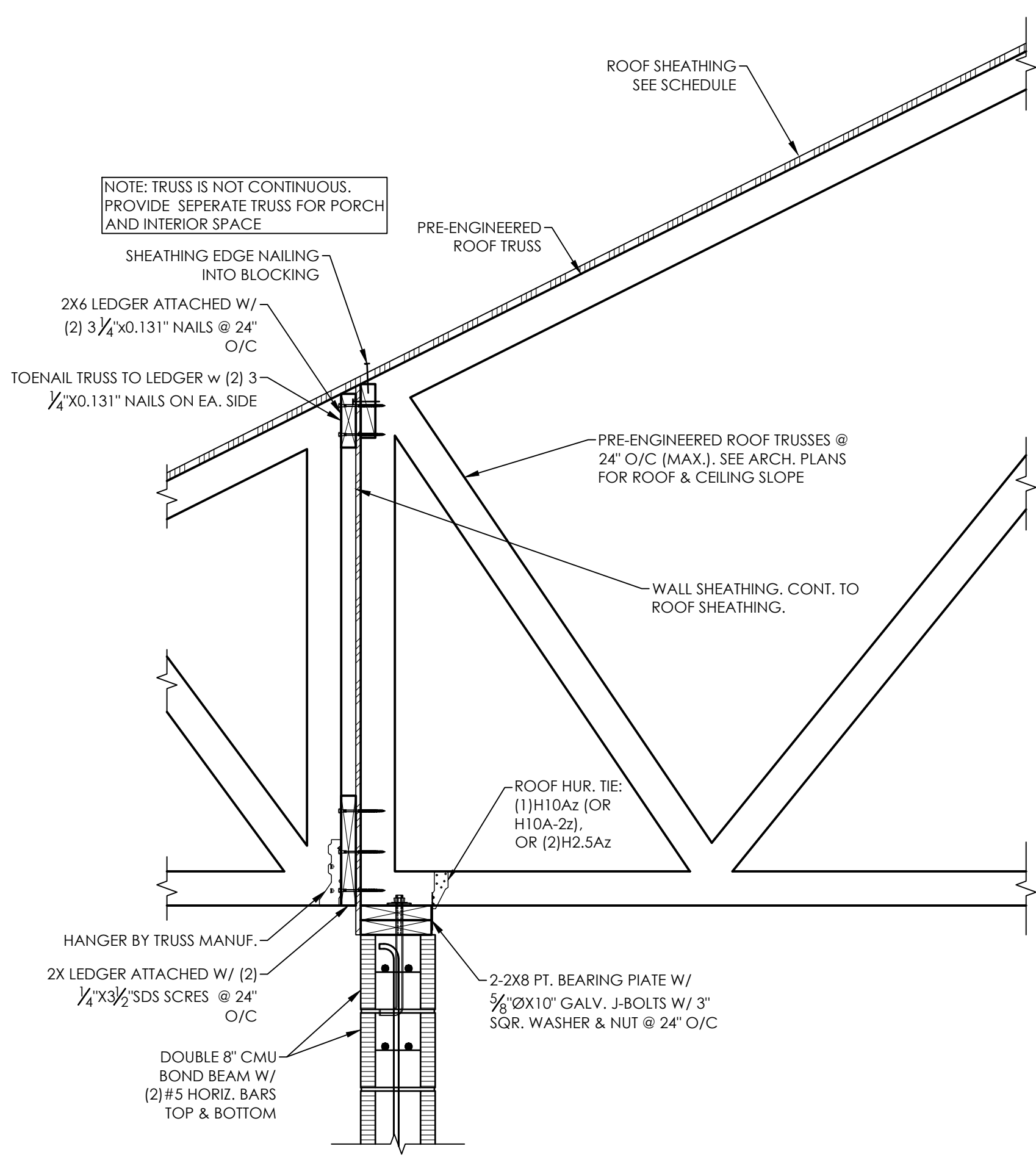
**4 RIGDE @ GABLE WALL**  
 SCALE: 1"=1'-0"



**4 ROOF TRUSS INT. BEARING**  
 SCALE: 1"=1'-0"



**5 ROOF TRUSS BEARING @ COLUMN**  
 SCALE: 1"=1'-0"



**6 TYP. FL. TRUSS BEARING DETAIL @ EXT. WALL @ PORCH**  
 SCALE: 1"=1'-0"



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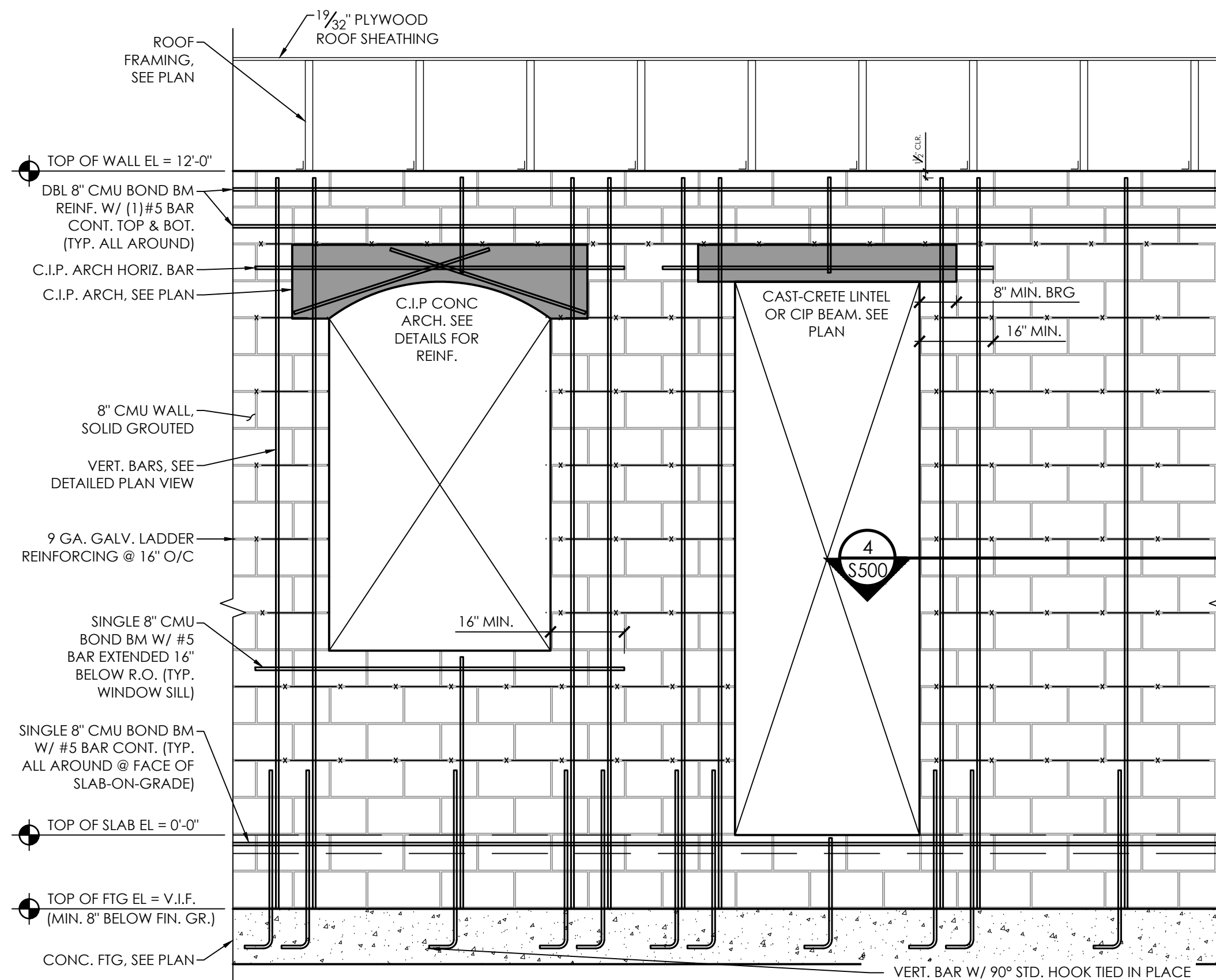
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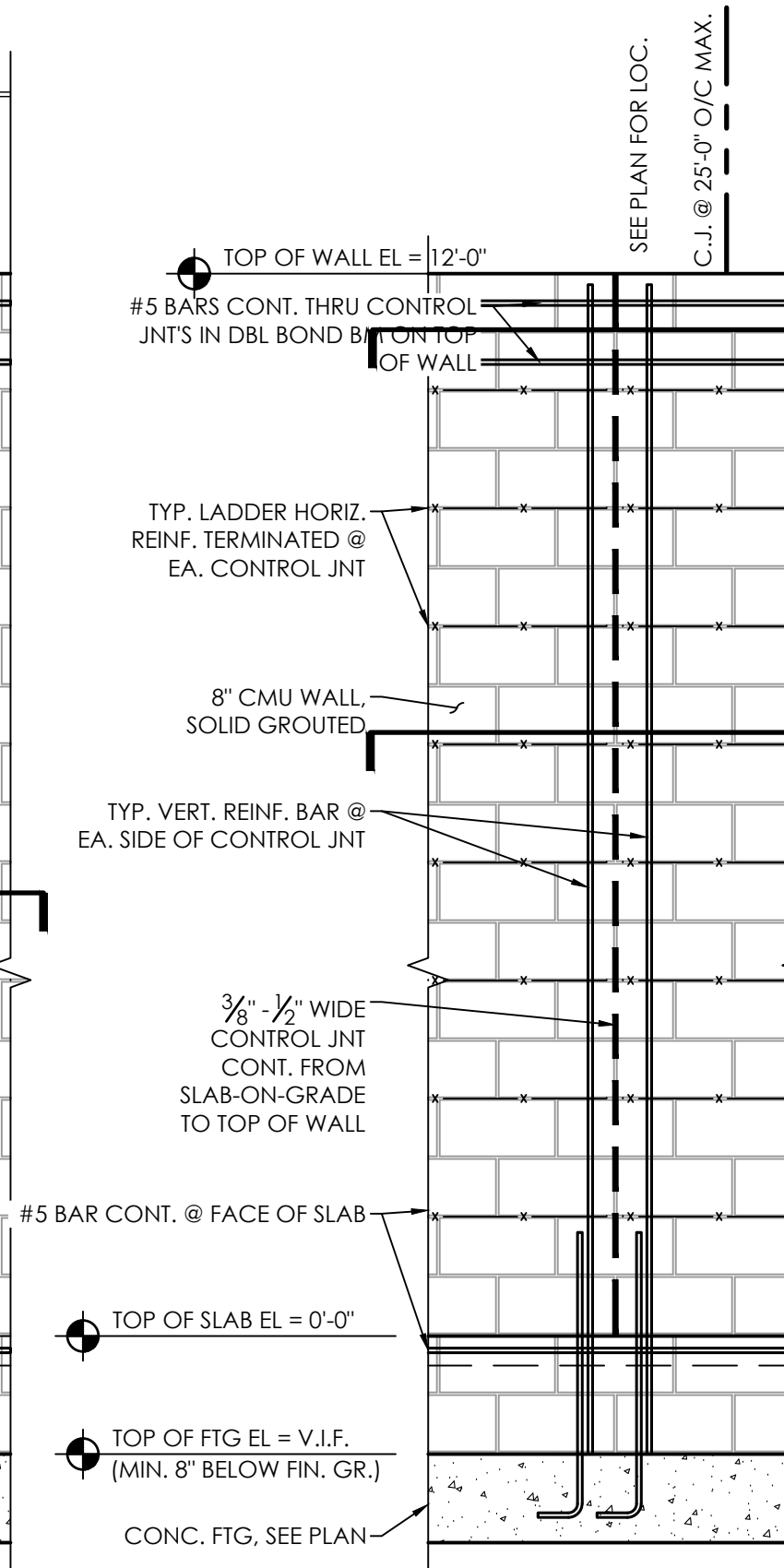
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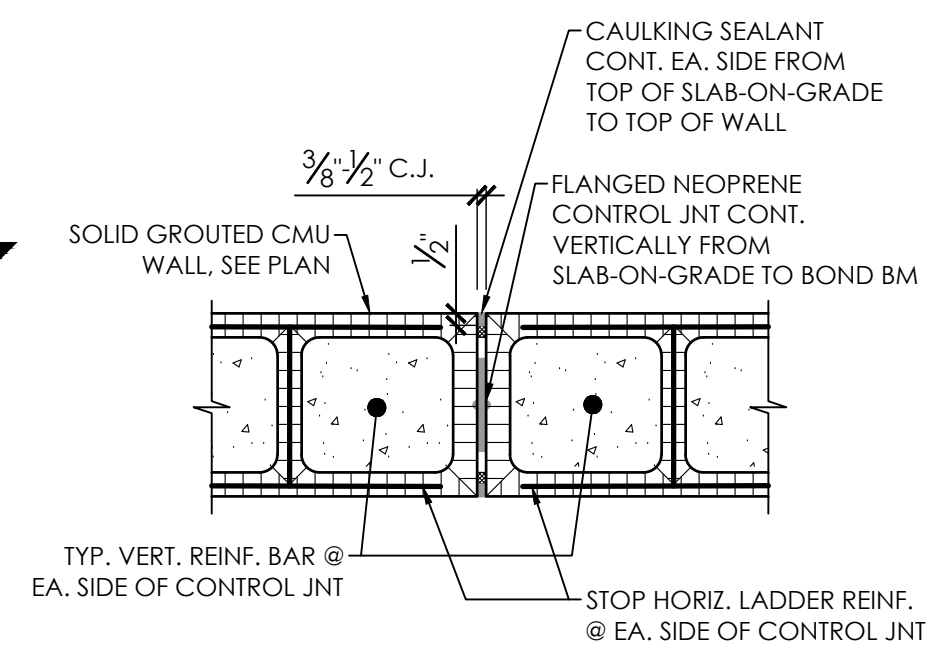
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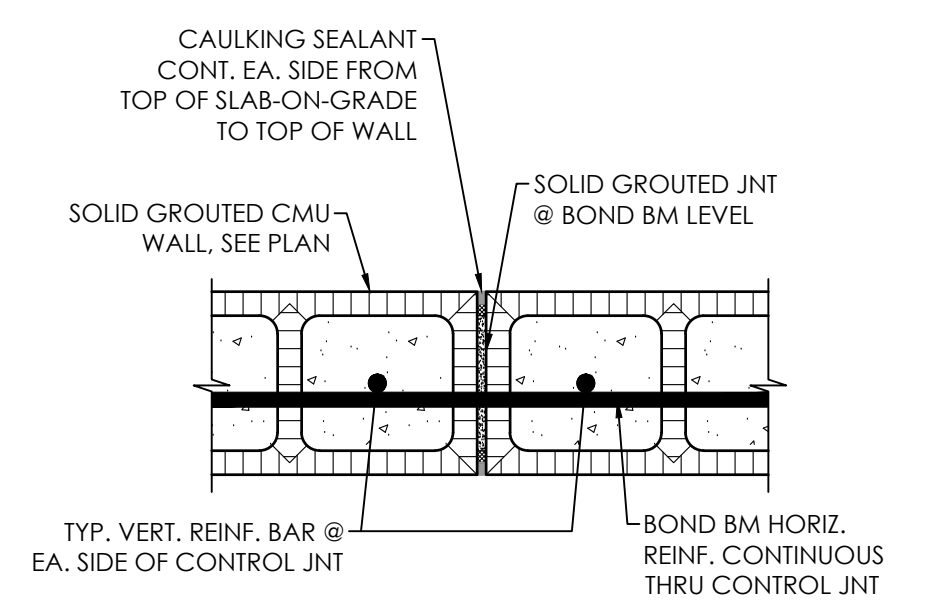
1 TYP. CMU WALL REINFORCING DETAIL. ELEVATION VIEW  
SCALE: 1/2" = 1'-0"



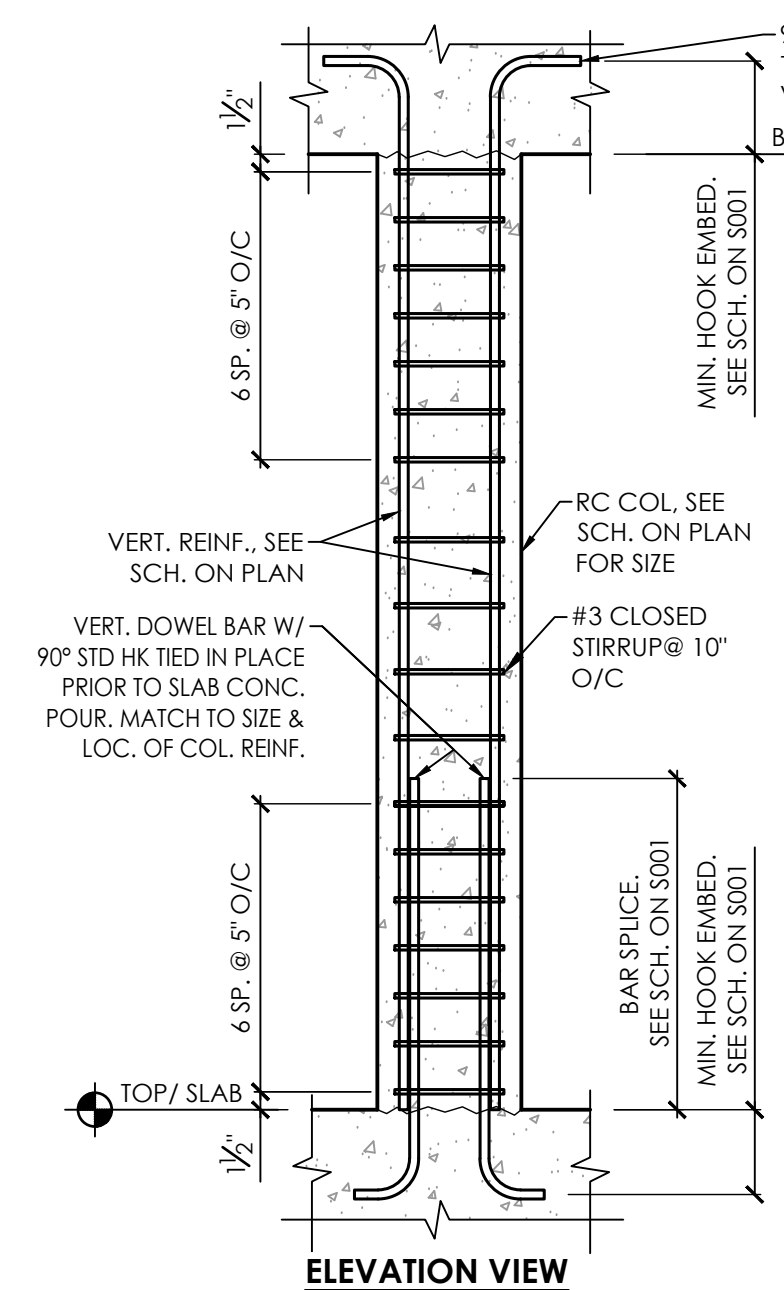
2 CMU WALL ELEVATION VIEW @ CONTROL JOINT LOC.  
SCALE: 1/2" = 1'-0"



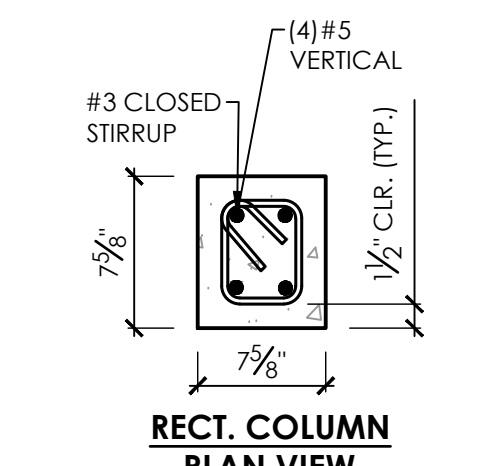
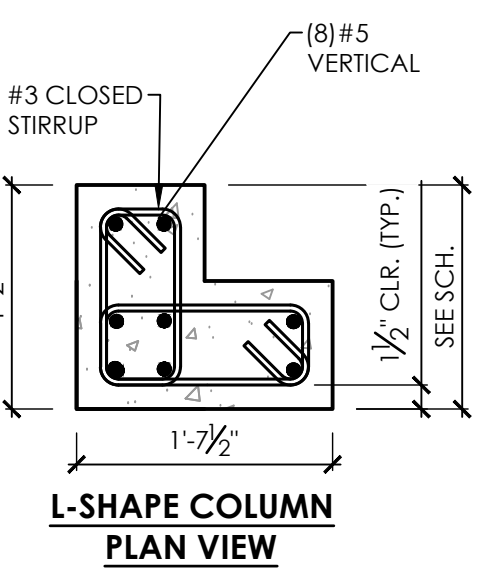
2-A CMU CONTROL JOINT DETAIL @ LADDER REINFORCING  
SCALE: 1-1/2" = 1'-0"



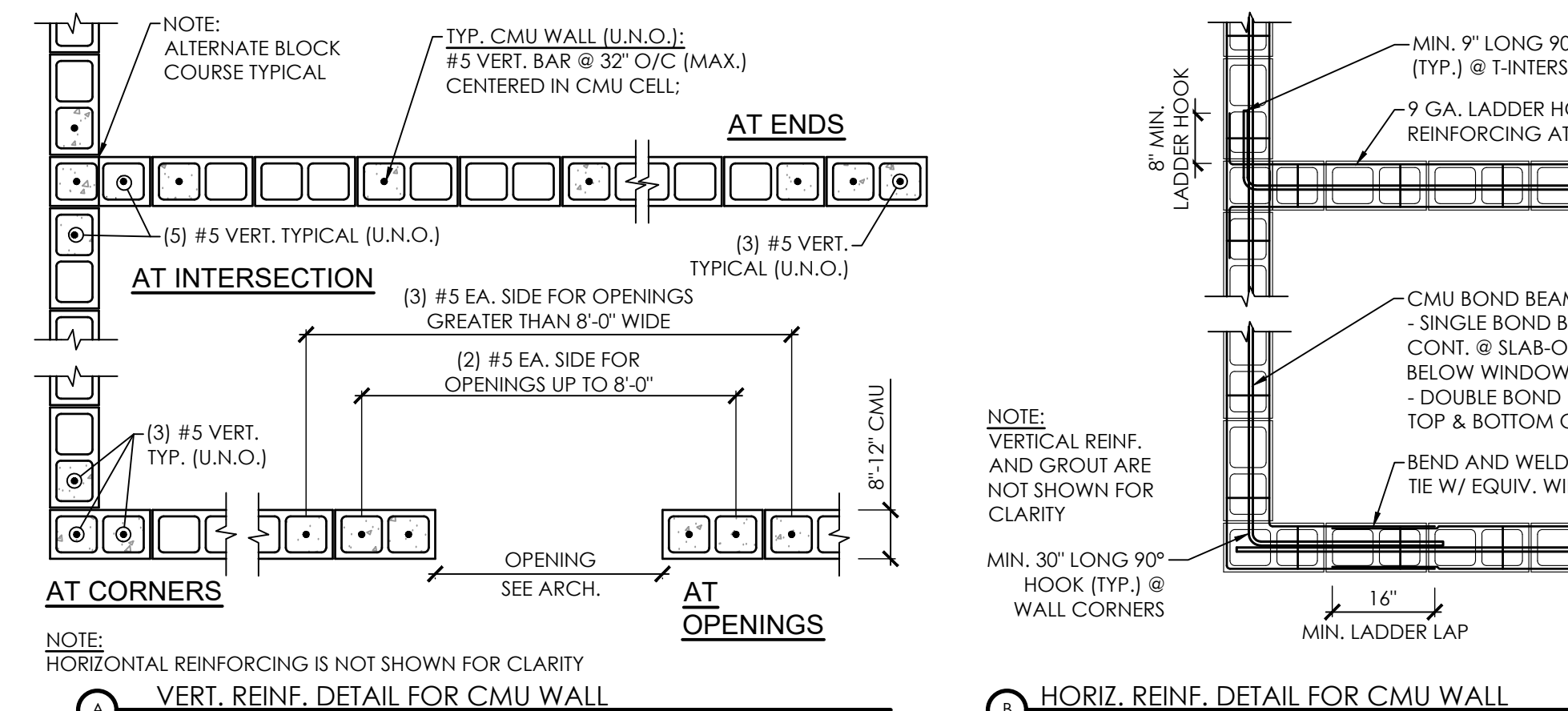
2-B CMU WALL ELEVATION VIEW @ JOINT LOC.  
SCALE: 1/2" = 1'-0"



3 TYP. CONCRETE COLUMN REINF. DETAIL  
SCALE: N.T.S.



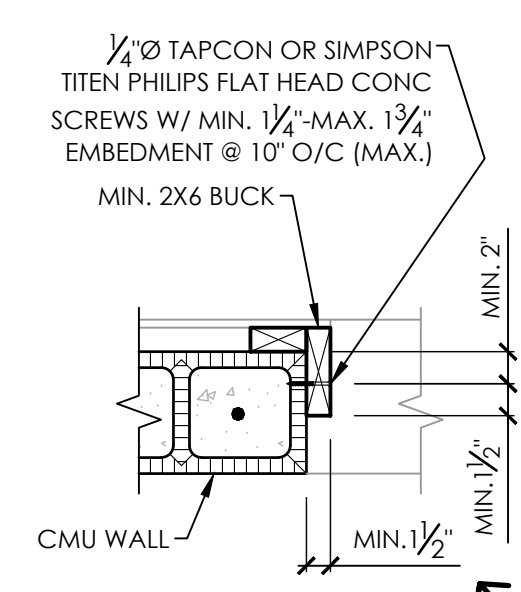
4 TYP. CONCRETE COLUMN REINF. DETAIL  
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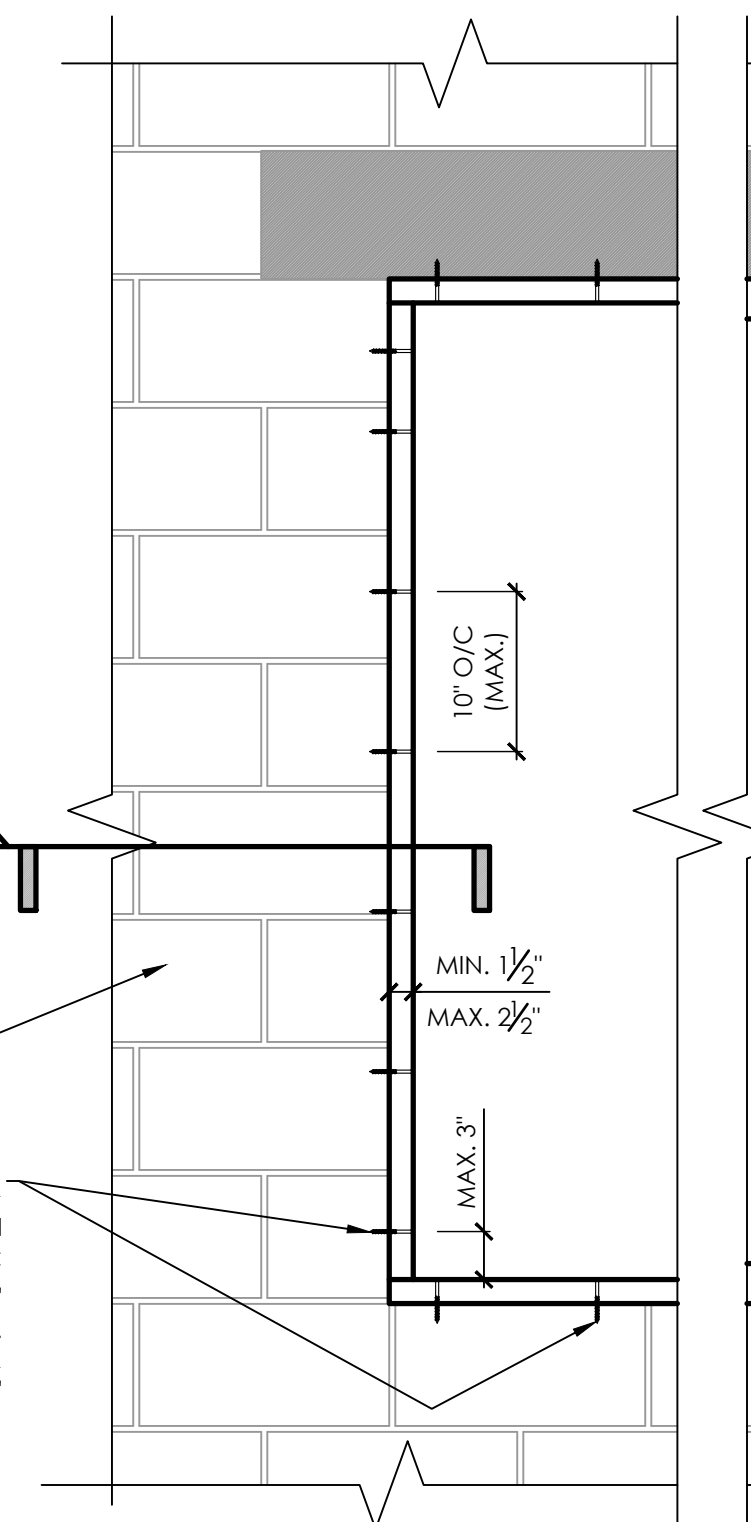
4 TYP. CMU WALL REINFORCING DETAILS. PLAN VIEW  
SCALE: N.T.S.



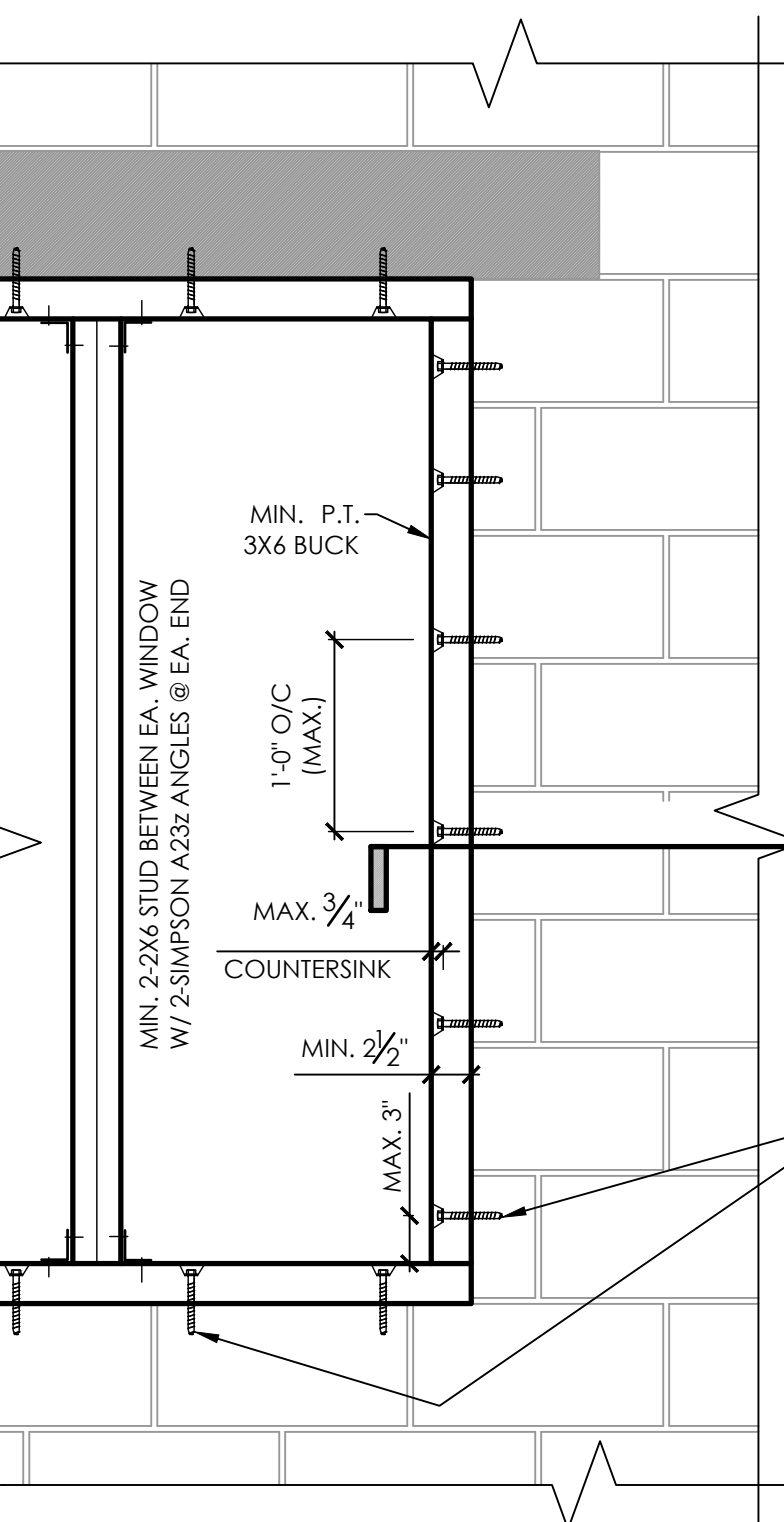
5 HORIZ. REINF. DETAIL FOR CMU WALL



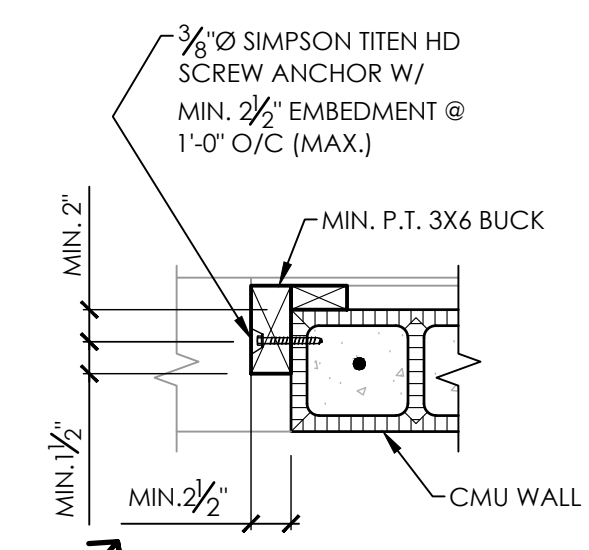
OPTION 1



5 TYP. WINDOW/DOOR BUCK CONNECTION DETAIL  
SCALE: 1" = 1'-0"



OPTION 2



OPTION 2

NOTE: ATTACH 2 1/2" THICK WINDOW BUCK USING 3/8" SIMPSON TITEN HD SCREW ANCHOR W/ MIN. 2 1/2" EMBEDMENT IN CONCRETE. NOTE: DISTANCE BETWEEN SCREW ANCHOR & EDGE OF CONC. SHALL BE MIN. 2"

NOTE: WOOD BUCK SHALL BE INSTALLED ONLY IF REQ'D PER ARCHITECT'S AND WINDOW/DOOR MANUFACTURER'S SPECIFICATIONS.



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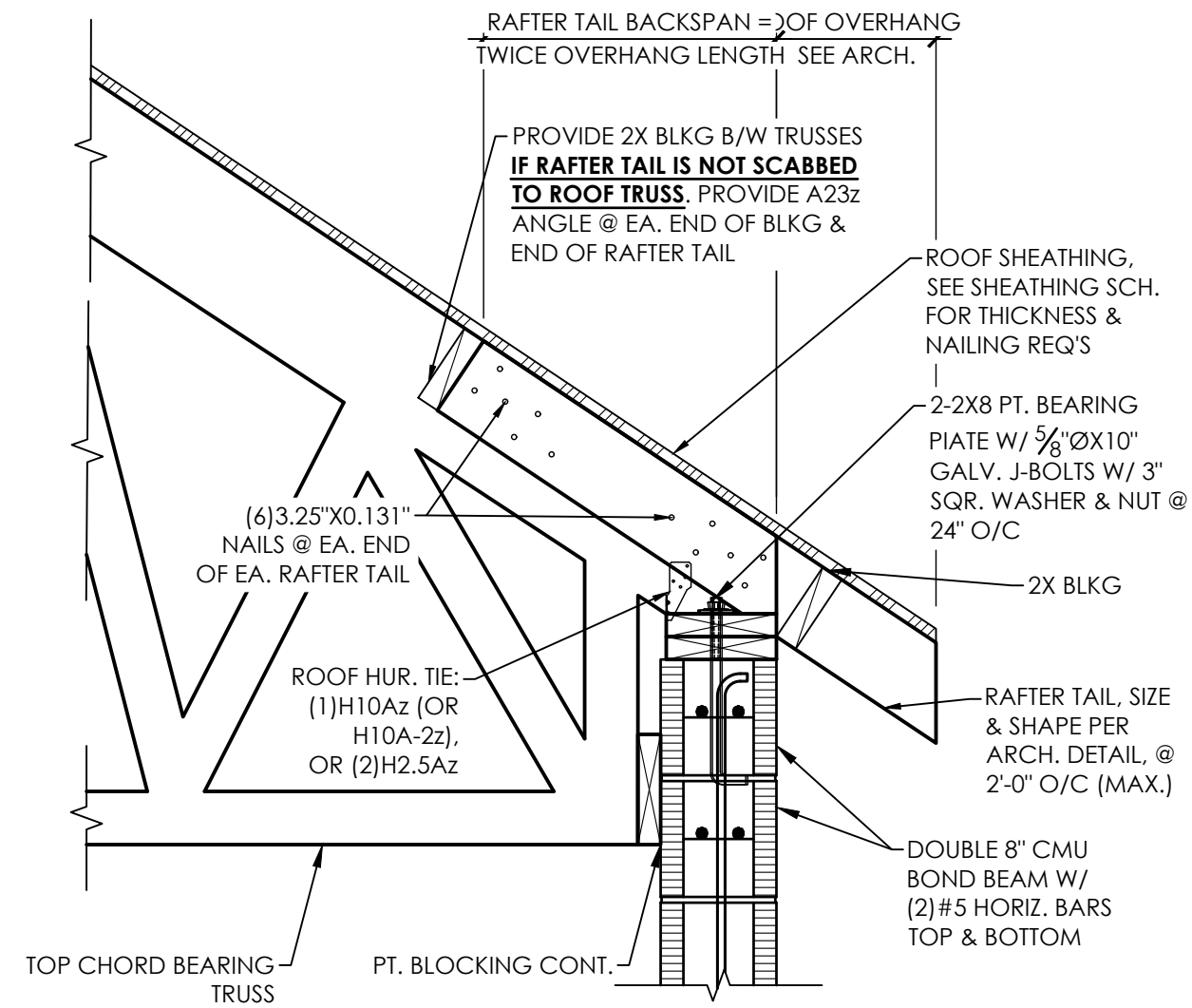
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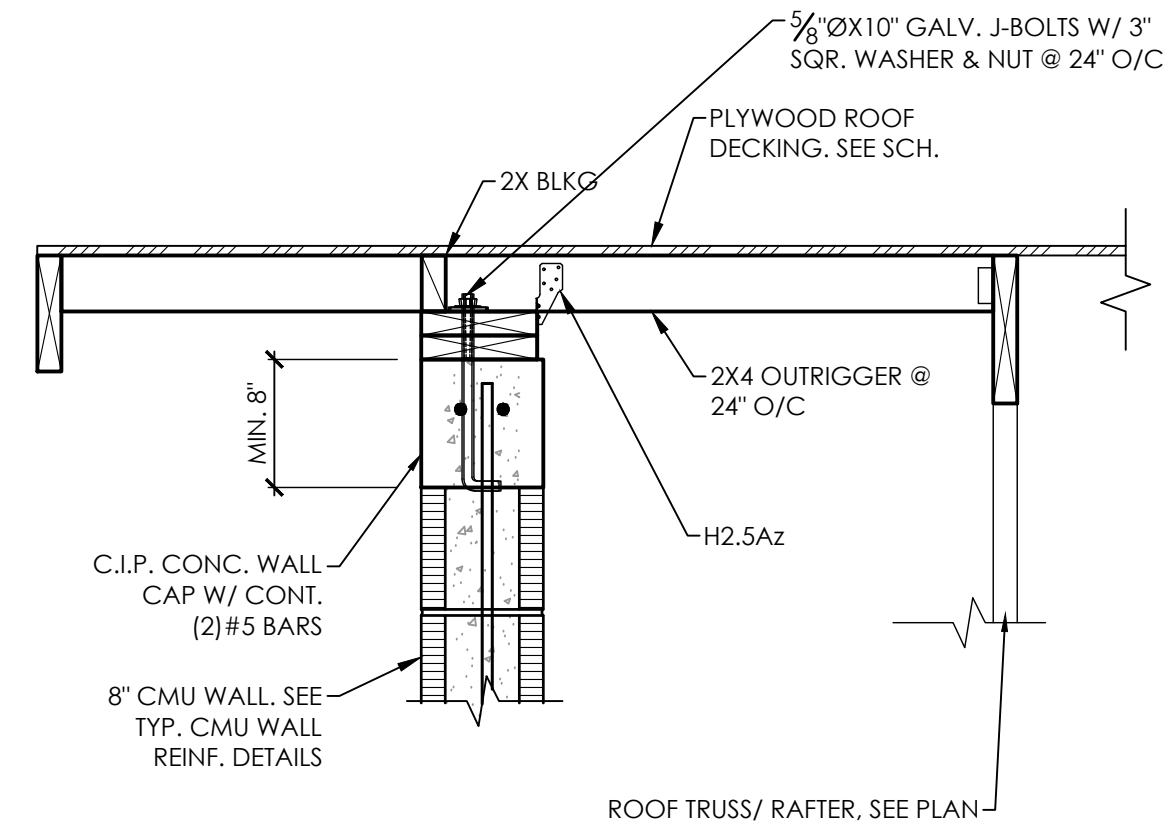
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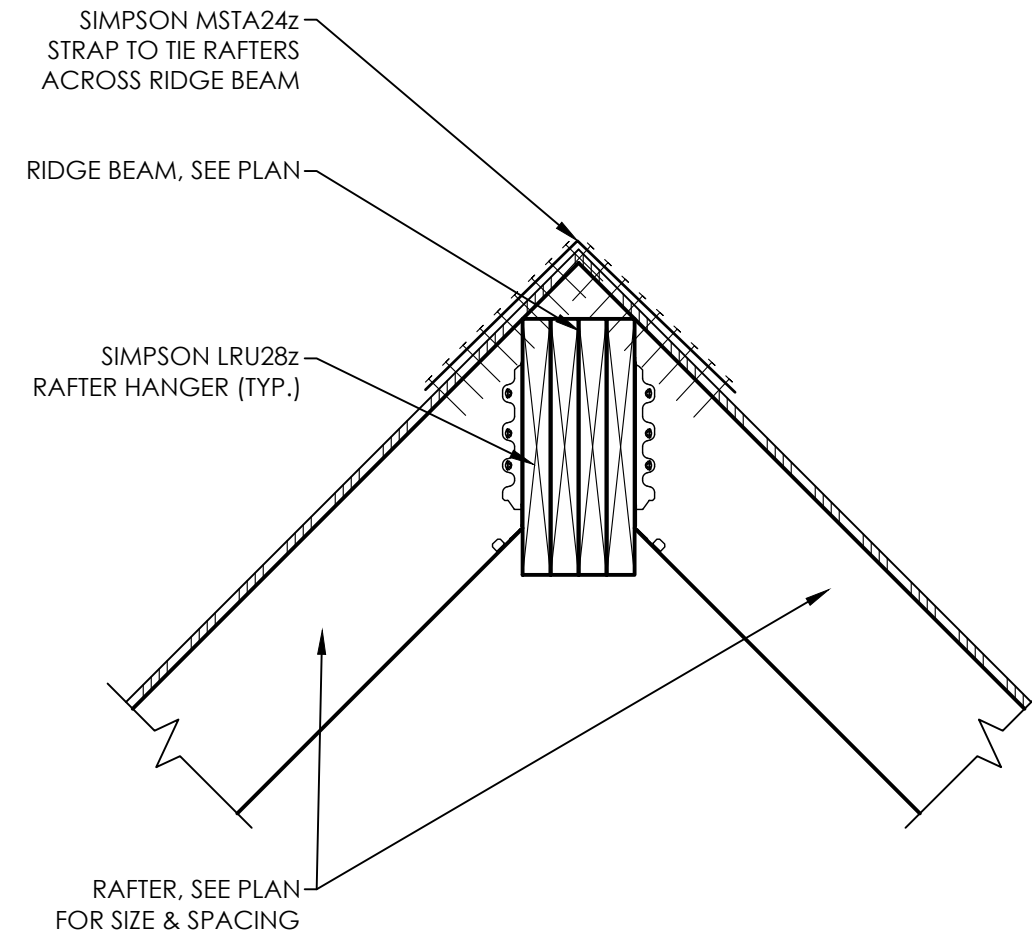
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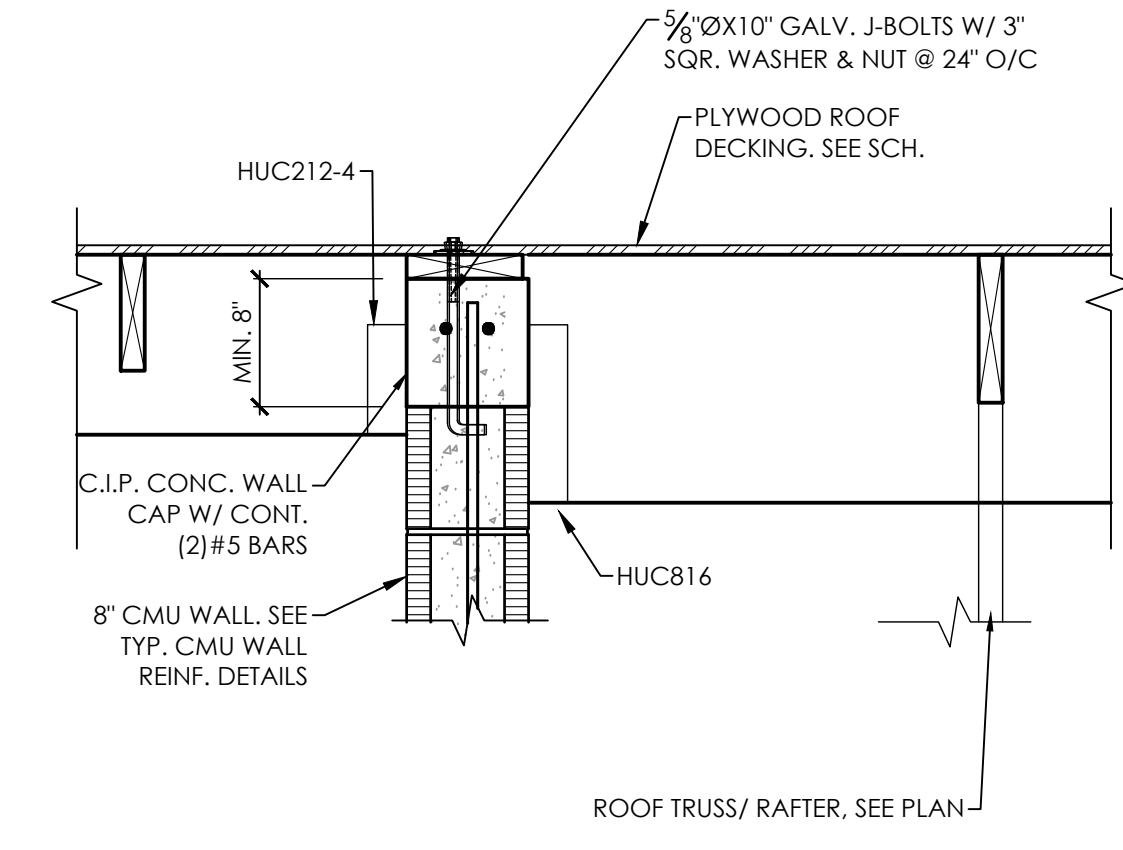
**1 TYP. ROOF TRUSS DETAIL**  
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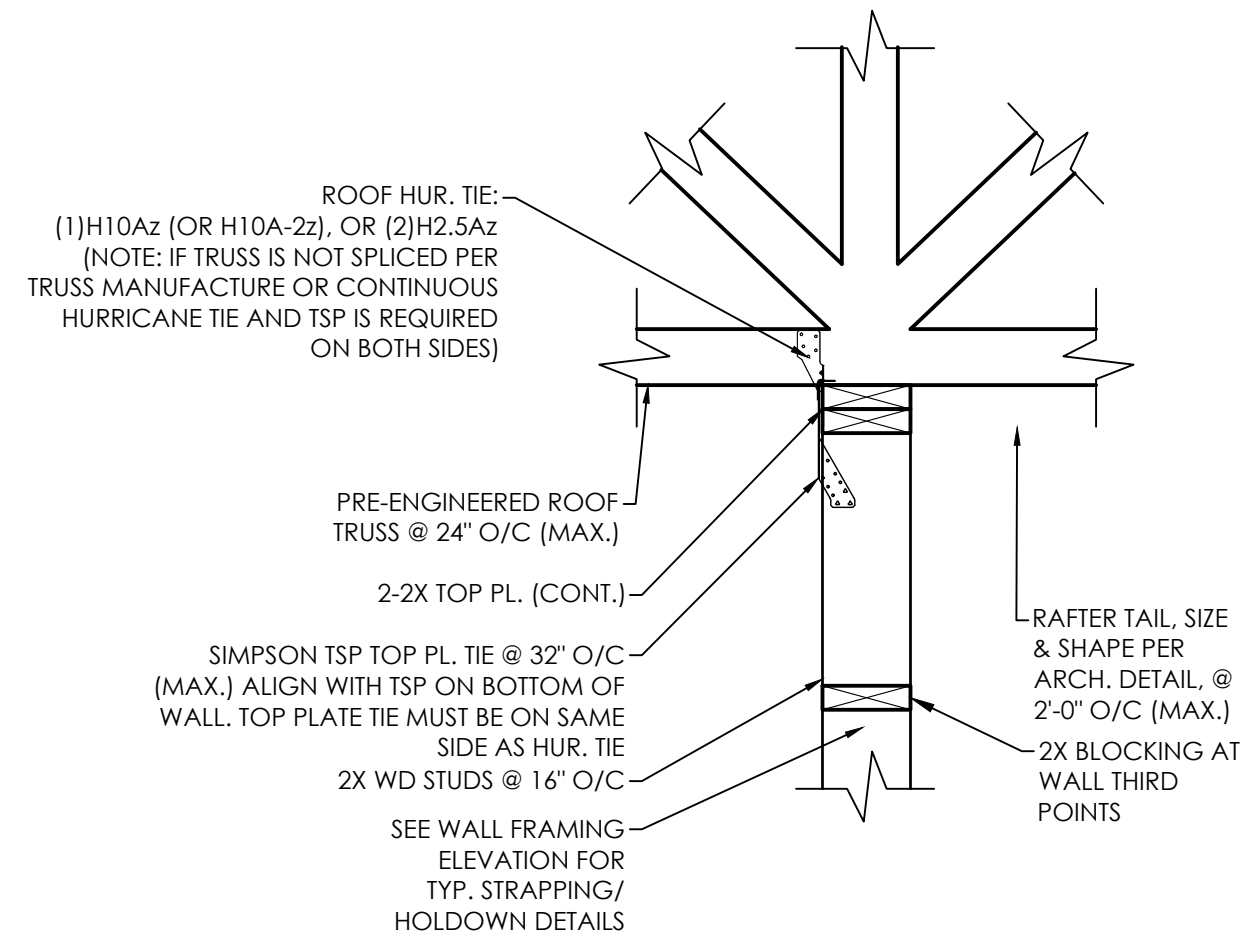
**2 TYP. GABLE END DETAIL**  
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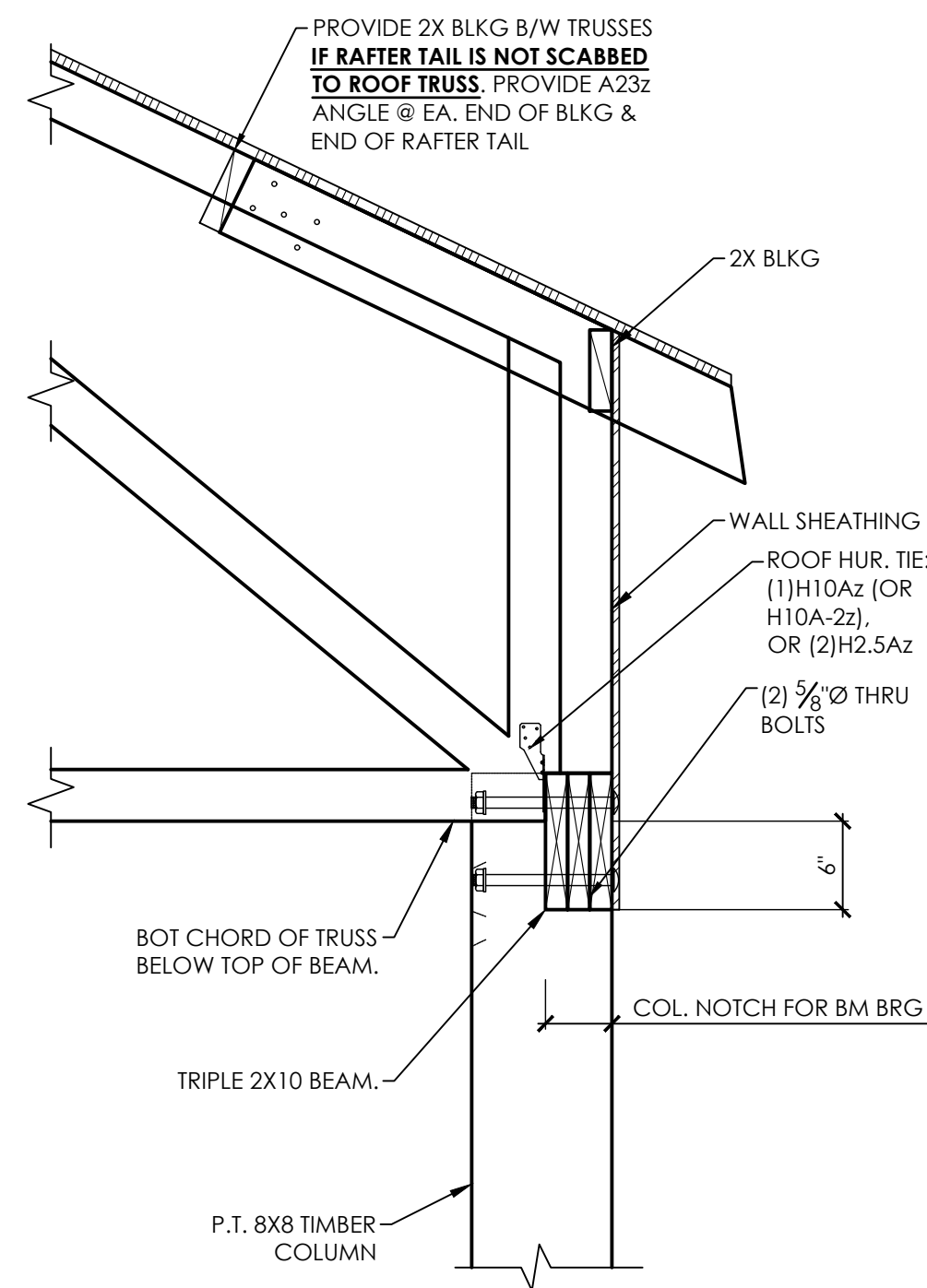
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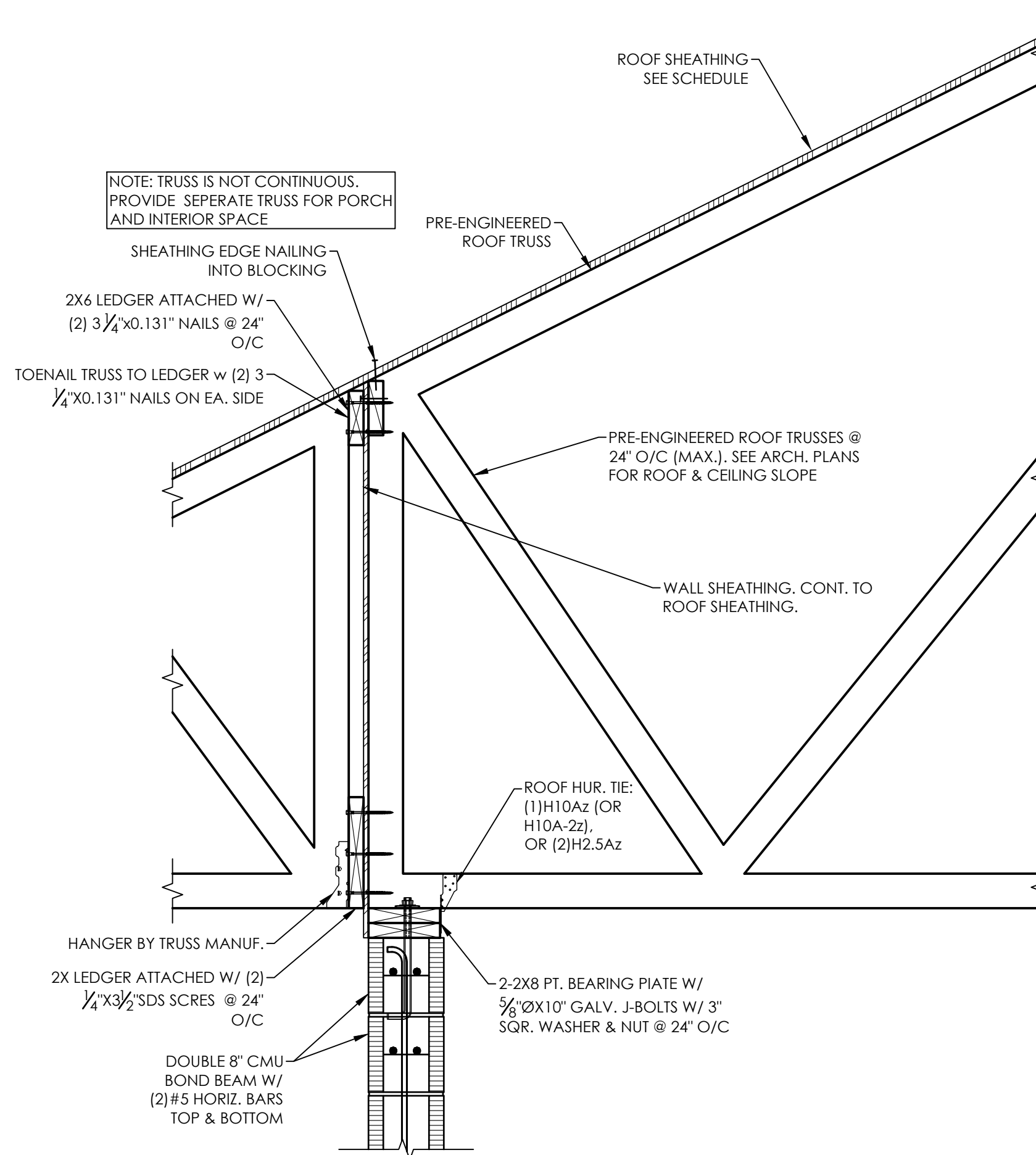
**4 RIGDE @ GABLE WALL**  
 SCALE: 1"=1'-0"



**4 ROOF TRUSS INT. BEARING**  
 SCALE: 1"=1'-0"



**5 ROOF TRUSS BEARING @ COLUMN**  
 SCALE: 1"=1'-0"



**6 TYP. FL. TRUSS BEARING DETAIL @ EXT. WALL @ PORCH**  
 SCALE: 1"=1'-0"



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PRELIM SET

**South Walton County Mosquito Control District  
 New Headquarters Building**

774 North County Highway 393  
 Santa Rosa Beach, Florida 32459

PRELIMINARY  
 NOT FOR CONSTRUCTION

No.	Description	Date

STRUCTURAL DETAILS

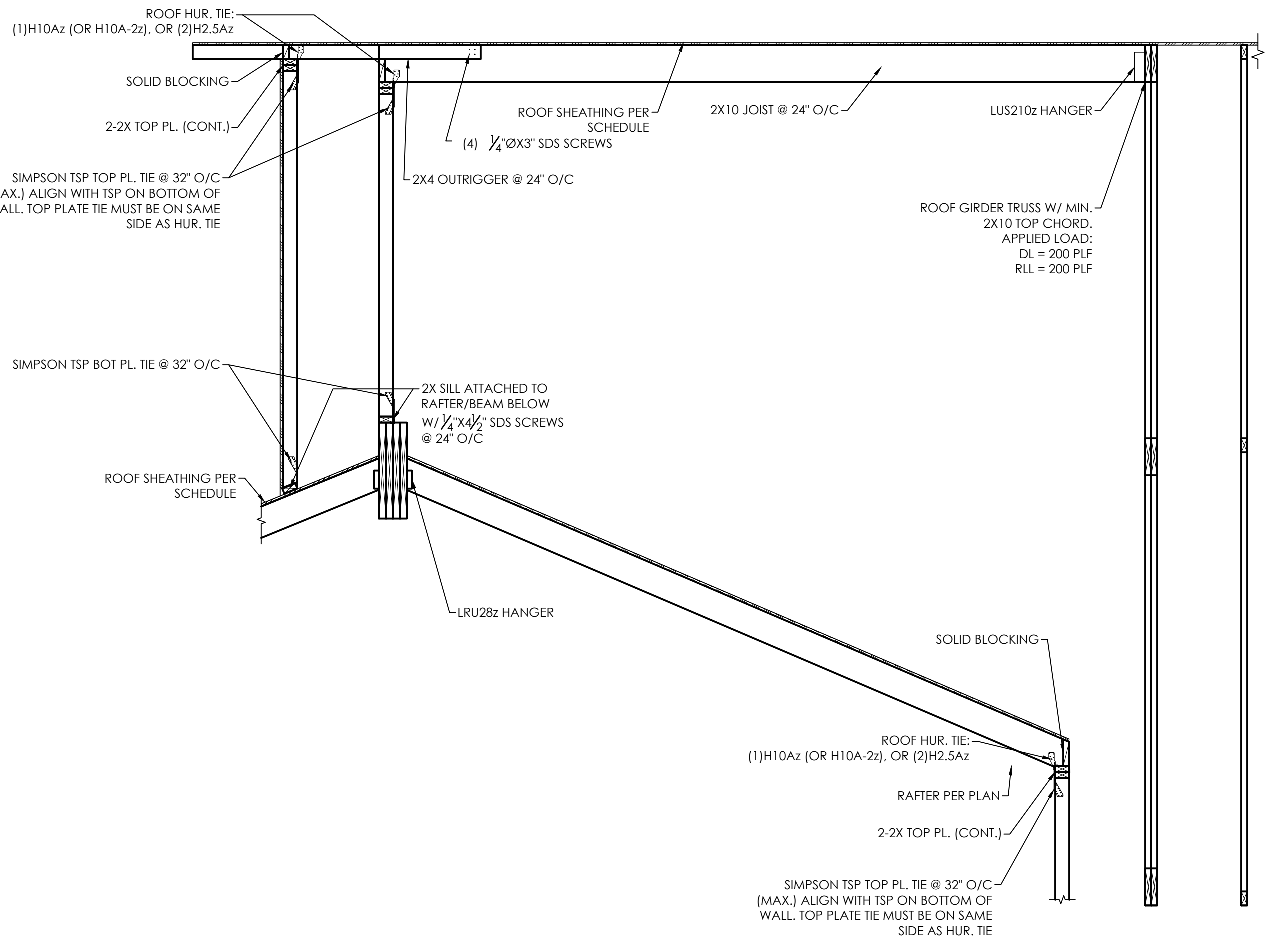
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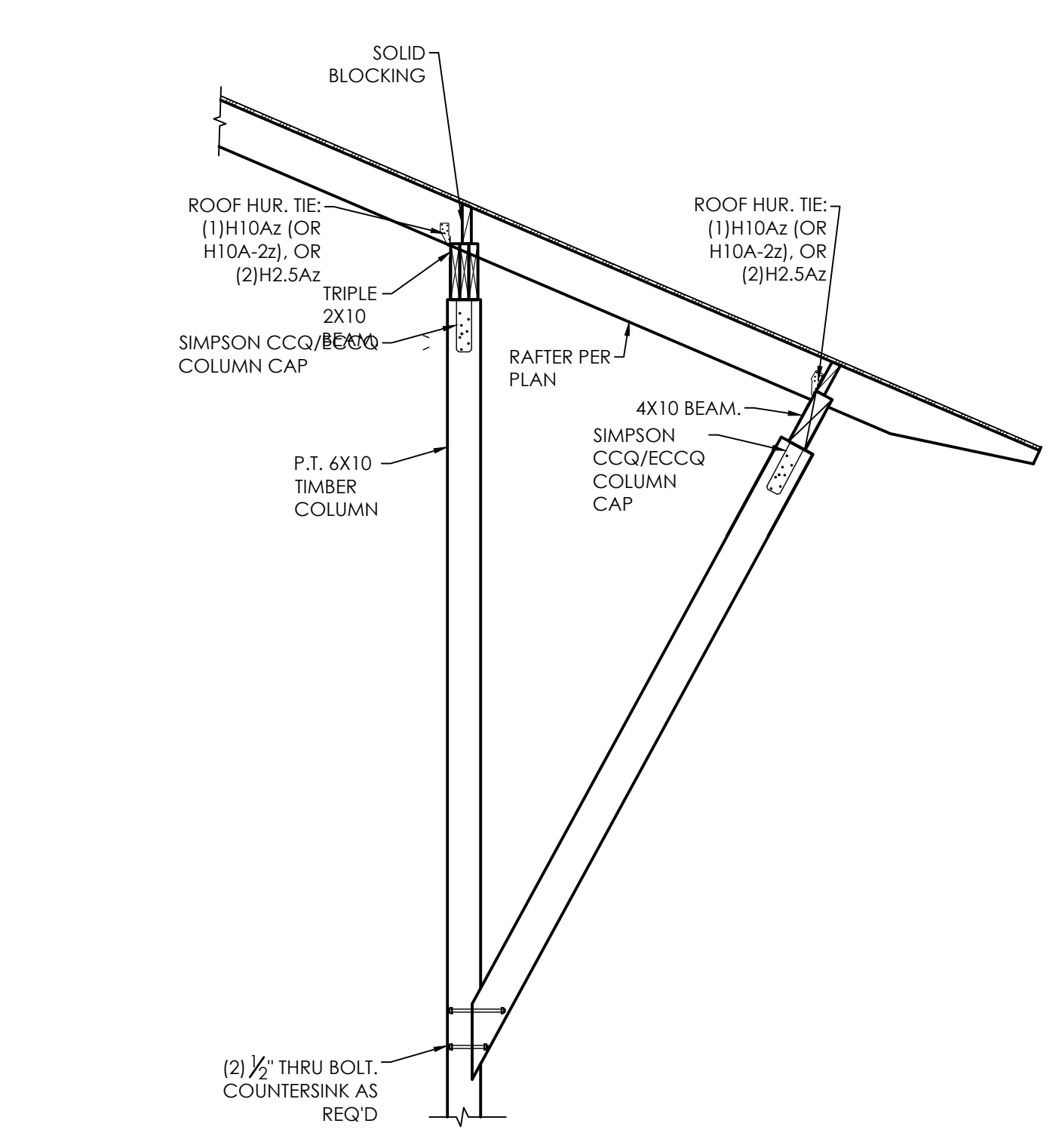
OA Project number: 18178  
 DAG Project number: 18106  
 Date: 08.02.23  
 PIC: NOC  
 PM: JMB

S502

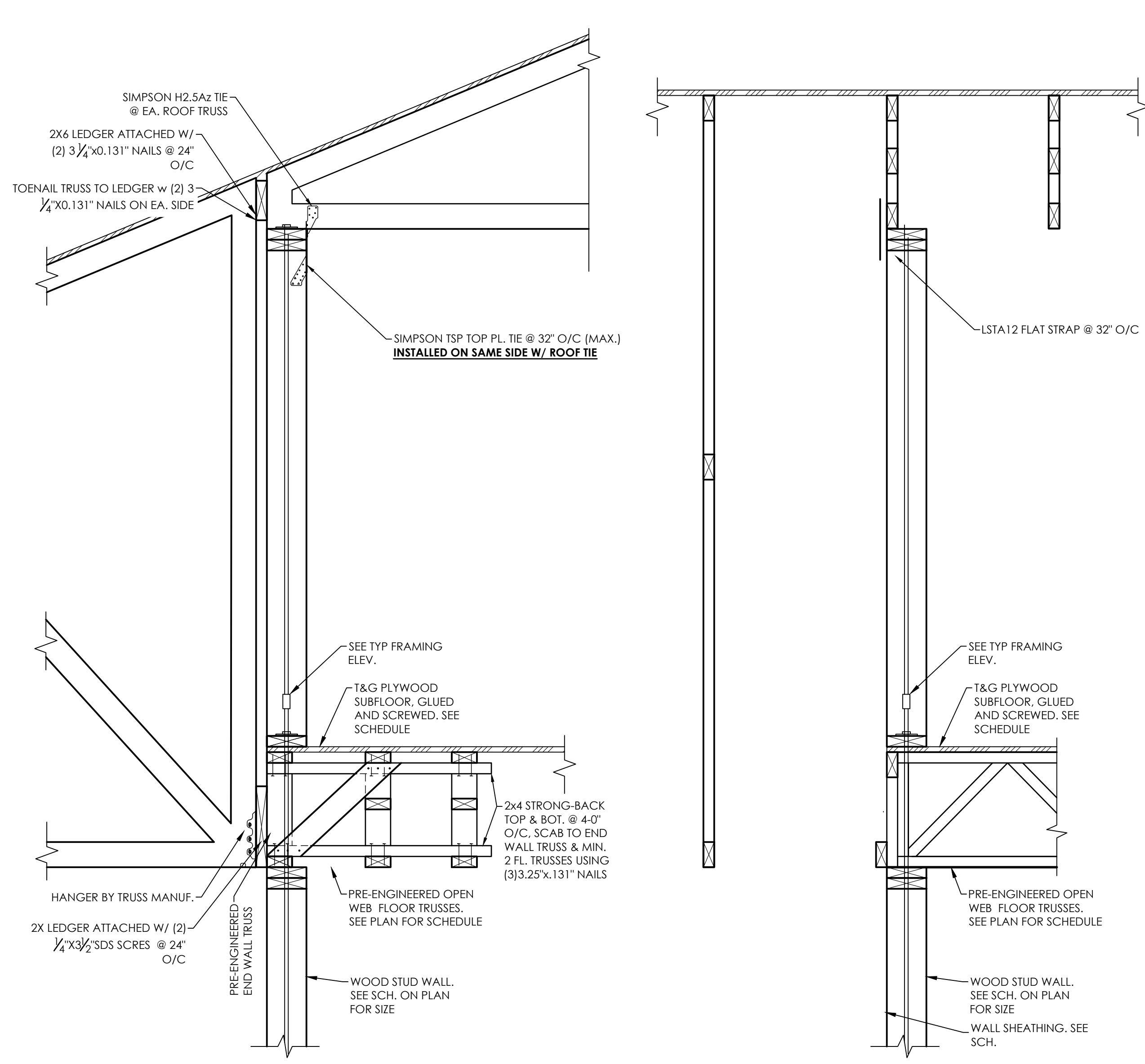
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**1 VAULTED ROOM FRAMING DETAIL**  
 S503 SCALE: 1"=1'-0"



**3 ENTRY BRACKET DETAIL**  
 S503 SCALE: 1"=1'-0"



**A TRUSS BEARING ON WALL**

**2 ELEVATED FLOOR FRAMING DETAIL @ STUD WALL**  
 S503 SCALE: 1"=1'-0"

**A TRUSS BEARING ON WALL**



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